Subdivision Development Investigation Casa Grande (NW & SW 26-35-5 W3M) Grasswood, Saskatchewan

File S1607

29 August 2008

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1.0 Introduction

1.1 Background

This report presents results of the investigation conducted for the proposed Casa Grande subdivision located south of Saskatoon on Preston Avenue. A site location plan is presented in Drawing No. S1607-01. The legal description of the area is NW and SW26-35-5-W3M. The proposed subdivision would consist of 70 - 80 lots ranging in size from 1 - 5 acres on both quarters being considered for development. The site is primarily used as pastureland. There are currently two residences located on each quarter section.

No previous geotechnical and hydrogeological investigations have been performed at the proposed Casa Grande subdivision development.

1.2 Objectives

The objectives of the subdivision investigation were to evaluate slope stability, determine wastewater disposal characteristics, and to provide preliminary foundation and construction recommendations based on a geotechnical investigation.

The investigation is to provide a preliminary assessment of site conditions which will be a first step in developing data to support subsequent applications to regulators. The level of detail is intended to provide basic site characterization. Further detail may be required for regulators such as Saskatchewan Health or Saskatchewan Environment.

1.3 Scope of Work

The scope of the investigation included:

- Compilation of local and regional geological information for the area;
- Assessment of the stratigraphy and hydrology at the site;
- Visual investigation of the site and aerial photograph analysis for evidence of slope instability;
- Preliminary geotechnical and hydrogeological investigations to assist with permit applications for installation and construction of wastewater disposal systems;

- Foundation recommendations and restrictions arising from the geotechnical investigation; and,
- Reporting, including stratigraphic cross-sections identifying the geology and definition of the piezometric surface of the site.

1.4 Existing Information

Various sources of information are available which were used to develop a general assessment of the geological and hydrogeological features of the subject site and its surrounding area. The following information was used for an assessment of the area around this site:

- Christiansen, E.A., 1970. Physical Environment of Saskatoon, Canada
- Saskatchewan Geomatics aerial photographs, 1990
- SaskWater Well Data provided by SaskWater

2.0 Physical Environment

2.1 Regional Geology

The bedrock surface in the region consists of the Cretaceous Bearpaw Formation which is overlain by a succession of Quaternary deposits of till and stratified drift from the Saskatoon Group. The Bearpaw Formation is the youngest bedrock in the area, and has a varying thickness near the study area of 45 to 62 m thick. It is a non-calcareous, silt and clay. The uppermost glacial deposit consist of the Saskatoon Group that includes the Floral and Battleford Formations and the surficial stratified drift deposits. In the area the Floral Formation is absent along with the Sutherland Group. The Saskatoon Group Formation is approximately 100 m to 110 m thick and consists of clay till. The surficial stratified drift deposits consist of stratified silt, clay, sands and gravels.

2.2 Regional Hydrogeology

The mapped aquifers are mainly surficial stratified deposits. The Moose Woods Flats Aquifer is the most extensive aquifer in the region. The aquifer at the site is approximately 37 m bgs. A search of the SaskWater Corporation Database (current to May 2007) indicated 133 water withdrawal well and five water test hole records within a one mile radius of the site. These

records provided a good sample of water use for the area. The majority of the boreholes were complete below 5.5 m bgs in surficial deposits and sand layers and lenses present in the Saskatoon Group. The borehole lithology logs showed that the stratigraphy in the area consisted primarily of sand, silt and clay layers.

3.0 Field and Laboratory Investigation

Subsurface geology was investigated by a total of 19 test borings on the site, designated as Bore Holes 101 to 119. The locations of bore holes are shown on Drawing No. S1607-02. Bore holes were drilled to a depth of 6 m to 20 m. Drilling was conducted on 17 January and 8 February 2008 using a truck-mounted Brat drill rig and 125 mm solid stem continuous flight auger. Bore holes were logged and sampled at a 1.5 m interval.

Piezometers were installed in Bore Holes 101, 104, 108, and 111. Water levels in piezometers were measured on 25 February 2008.

Moisture contents were determined for all samples, and Unified Soil Classifications and particle size analyses were performed on select representative samples.

Observations made during the field investigations, visual descriptions and the results of laboratory tests are recorded in the Bore hole Logs and Summary of Sampling and Laboratory Test Data, and are appended to this report. An explanation of the symbols and terms used in the bore hole logs is included in the Symbols and Terms section of this report.

3.1 Stratigraphy

The site is located on a glacial lake basin, with subsurface soil consisting primarily of sand and silt with some clay. Some dune sand is present along the southern edge of SW26.

Stratigraphy consisted of stratified sand, silt and clay. Table 3.1 provides a summary of index properties of soil encountered, including moisture content and results of Atterberg limits and Unified Soil Classification testing. Silty sand covered the majority of the site to varying depth. Sand was generally moist and compact, with standard penetration testing 'N' values of 9 to 16 blows for 300 mm penetration.

Clay strata varied in thickness, and generally possessed medium to high plasticity. It was moist and stiff to very stiff in consistency, with an undrained shear strength of about 160 kPa.

Silt and clayey silt strata with some sand were encountered in some areas.

Sample Tested	Natural Water Content (%)	Plastic Limit	Liquid Limit	Plasticity Index	Unified Soil Classification*
BH101 @ 0.8 to 1.0 m	5.3	NP	NP	NP	SM
BH101 @ 1.5 to 1.8 m	25.8	23.9	68.2	44.3	CH
BH101 @ 2.3 to 2.4 m	9.8	NP	NP	NP	SM
BH101 @ 4.6 to 4.8 m	17.8	NP	NP	NP	SM
BH101 @ 7.5 to 7.6 m	34.0	23.6	57.9	34.3	СН
BH101 @ 10.6 to 10.7 m	38.9	28.7	77.9	49.2	СН
BH101 @ 12.1 to 12.2 m	34.6	24.0	74.6	50.6	СН
BH104 @ 0.7 to 0.8 m	9.2	NP	NP	NP	SM
BH104 @ 1.4 to 1.5 m	11.8	19.5	43.3	23.8	CL
BH104 @ 2.2 to 2.3 m	16.5	18.9	49.1	30.2	CL
BH104 @ 3.1 to 3.4 m	23.4	26.4	63.7	37.3	CH
BH104 @ 4.6 to 4.8 m	9.1	NP	NP	NP	SM
BH104 @ 7.5 to 7.6 m	28.7	NP	NP	NP	SM
BH108 @ 1.4 to 1.5 m	36.0	NP	NP	NP	SM
BH108 @ 2.2 to 2.3 m	6.7	NP	NP	NP	SM
BH108 @ 3.0 to 3.1 m	22.4	19.9	33.7	13.8	CL
BH108 @ 6.6 to 6.7 m	35.1	16.4	51.9	37.3	СН

Table 3.1Index Properties of Representative Samples

*CL - low plasticity clay, CH- high plasticity clay, SM - silty sand, NP- non-plastic

3.2 Groundwater

Groundwater levels were measured on 25 February 2008. The water elevation in each piezometer is presented in Table 3.2 and on Drawing No. S1604-02. Groundwater elevations were utilized to determine the horizontal hydraulic gradient. The groundwater flow direction at the site was determined to be towards the southeast.

Piezometer	Water Elevation (m)	Ground Elevation (m)	Casing Elevation (m)	Depth to Water (m)
BH101	482.572	487.448	488.412	4.88
BH104	482.262	489.012	489.896	6.75
BH108	482.400	486.440	487.390	4.04
BH111	485.264	487.281	488.094	2.02

Table 3.2 Water Elevations

4.0 Slope Stability

Landforms in the area were defined on the basis of aerial photography. An aerial photograph showing the site is presented in Drawing No. S1607-2. This area was part of Glacial Lake Saskatoon during the last deglaciation. While under water, sand, silt and clay was deposited. Sand dunes are present along the southern edge of the area being developed. The site has approximately 5 m of relief. Large scale landsliding is not an issue in this area.

5.0 Wastewater Disposal

5.1 Scope

The site was assessed in terms of the geotechnical and hydrogeological site characteristics required to install wastewater disposal systems as per the regulations and guidelines set out in the *Saskatchewan Onsite Wastewater Disposal Guide* (First Edition, 2007, Saskatchewan Health), and the *Onsite Wastewater Management: Review Process for Developments and Subdivisions* (Saskatoon Health Region, Public Health Services). These documents will be referenced as SOWDG and OWM, respectively, for the purposes of this report.

5.2 **Regulatory Requirements**

The proposed development falls within the High Sensitivity Area section as per the OWM. As such, only holding tanks, pressure chamber systems, package sewage treatment plants and Type II Mounds will be permitted at the Casa Grande site due to the proposed size and number of lots on each quarter section. It also states that any existing wastewater disposal systems in use or intended for use would need to be upgraded to comply with current requirements for high sensitivity developments.

The SOWDG states that there should be a minimum isolation distance of 1.5 m between a wastewater disposal system and the water table. All setback requirements listed in the SOWDG must also be met. Replacement disposal areas, if ever needed, should be located adjacent to the existing disposal area.

The SOWDG states that a Type II mound may be constructed on a natural slope provided that:

- The slope is less than 3% and the percolation rate is not slower than 60 minutes per 25 mm to a depth of at least 600 mm below the sand layer;
- The slope is less than 6% and the percolation rate is not slower than 30 minutes per 25 mm to a depth of at least 600 mm below the sand layer;
- The slope does not exceed 12% regardless of percolation rate.

A chamber system may also be constructed on a slope provided that distribution devices or step-downs are used.

Package sewage treatment plants such as three-cell septic tanks provide a greater level of treatment therefore may allow for a reduction in the treatment area required; however, they are not mandatory in a High Sensitivity Area.

5.3 Soil Loading Rates

As per the SOWDG, wastewater disposal systems are sized based on the soil loading rate. Soil loading rates are determined either via percolation testing or soil texture classification. Soil texture classification was used to determine appropriate soil loading rates for the various surficial soil types encountered at the proposed Casa Grande subdivision.

5.3.1 Soil Texture Classification

The results of the particle size analysis and hydrometer testing performed on select samples that are representative of the material in the upper strata are presented in Table 5.1. Based on the percentage of silt and clay versus sand, the soil was classified as per the Soil Texture Classification Triangle in Appendix 15 of the SOWDG.

Soil Type	Sample Number	Gravel (%)	Sand (%)	Silt (%)	Clay (%)	Soil Texture as per SOWDG
Sand	KB13	0.0	76.7	13.2	10.1	Sandy Loam
Silty Clay	KB41	0.0	14.9	16.3	23.8	Silt Loam
Clay	KB82	0.0	25.4	36.2	38.5	Clay Loam
Silt	MN19	0.0	32.3	50.4	17.2	Loam
Sand	MN25	0.0	74.6	12.6	12.8	Sandy Loam

Table 5.1Summary of Particle Size Analysisand Soil Texture Classification of Upper Soil Units

The corresponding loading rates as per Appendix 15 of the SOWDG can be applied to size the wastewater disposal systems:

•	Clay Loam (Clay)	10.78 L/m^2
•	Silt Loam (Silty Clay)	13.72 L/m ²
•	Loam (Silt)	17.15 L/m ²
•	Sandy Loam (Sand)	22.05 L/m ²

It is important to note that the soil texture classifications provided in this report are based upon a single hydrometer test for each surficial soil unit encountered. Also, the classifications do not account for secondary structure within the soil unit such as fracturing, which can greatly increase the permeability of a soil.

5.4 Groundwater

Water level measurements taken at the site (Table 3.2) indicate that the water table at the site ranges from 6.75 m to 2.02 m below ground surface, which ranges in elevation from 485.26 m to 482.26 m.

Assuming an average water table location of about 2 to 5 m below ground surface, the minimum separation distance of 1.5 m will be met.

A water sample was taken from the standpipe piezometers in BH101 and 108 and analyzed for routine water chemistry to obtain background groundwater data. The results are presented in Appendix A.

5.5 Conclusions

The following recommended wastewater disposal systems for the proposed development are described in relation to the soil type of the disposal area:

- Type II mound systems with two-cell septic tanks would provide adequate wastewater disposal for areas where the surficial material is sand, silt, or clay.
- Type II mound systems, with three-cell septic tanks would provide adequate wastewater disposal for areas where the surficial material is sand, silt, or clay.

The soil loading rate used to size the selected system should be determined based on the lowest soil loading rate of any of the materials encountered in the upper 900 mm of soil (excluding topsoil).

6.0 Foundation Design and Construction Recommendations

It is our understanding that the subdivision will primarily consist of single family dwellings. Structures will likely be one or two storeys over a full basement with a grade supported concrete floor. Geotechnical issues associated with this type of structure are foundations to support the proposed structures and construction of a grade supported floor on a variable subgrade that could be silt, sand or high plasticity clay. Groundwater levels measured were 2.0 m to 6.8 m below existing ground surface (Table 3.2), and vary in elevation from 485.26 m to 482.26 m. Groundwater levels can be expected to vary with time, and may increase as a result of development.

Although fill material was not noted during this field investigation, its presence and condition should be noted during construction since it is not desirable to place foundations or floors on fill material of unknown composition and consistency. Foundations or floors should not be constructed on organic topsoil or organic soil.

6.1 Waterproofing and Subdrainage

The quantity of seepage and groundwater levels will vary seasonally, with precipitation or snowmelt, and with development due to irrigation and other factors. Rates of flow can be relatively high through sand and sandy strata. Groundwater levels are variable across the site, and were at least 2.0 m below ground surface.

Typically, basement floors will be about 1.5 m to 2.0 m below finished grade. On this basis, it is not likely that hydrostatic pressures will develop on basement walls and floors. However, basement walls can be waterproofed to accommodate any future increases in groundwater levels that could lead to seepage into basements. At a minimum, walls must be damp-proofed; floors should be damp-proofed.

A perimeter subdrainage system should be installed at the base of the footing. This requirement can be reviewed depending on specific conditions for any home. The excavation should be backfilled with a free draining granular soil to within about 0.6 m of surface to ensure that water can freely drain to a perimeter weeping tile system. Free draining means that there is less than 3 percent silt and clay particles. Clay or clayey soil can be placed on the surface to reduce the amount of infiltration.

6.2 Foundations

The proposed structures may be supported on shallow spread footings or augered cast-in-place concrete piles. Lightly loaded structures supported on shallow spread footings on medium to high plasticity clay will experience some vertical movement associated with changes in soil moisture. Total vertical movement is estimated to be as much as 150 mm for foundations on medium to high plasticity clay, with differential movement less than half of the total vertical movement. The anticipated vertical movement for foundations on sand will typically be less than about 25 mm.

Assuming that the basement or crawlspace will be insulated, the footing must be constructed below the anticipated depth of frost, estimated to be about 1.8 m in this area. This depth can be reduced on the basis of local experience and on the performance of similar foundations in the area. The foundation should not be allowed to freeze, particularly during construction, as frost heave may occur.

The allowable bearing capacity for a shallow spread footing will depend on the type of soil at the footing elevation. The information provided in this report is for preliminary purposes, only, and should not be relied upon for detailed design due to the variation in conditions across the site. Site specific investigations are recommended for buildings on this development. Foundation conditions, soil type, and allowable bearing capacity should be confirmed for specific sites. For preliminary design, the allowable bearing capacity for a spread footing constructed on compact sand will be 100 kPa (2,100 lb/ft²). For shallow spread footings constructed on stiff clay, the same value can be used. Sand encountered at the foundation elevation should be well compacted to minimize the potential for settlement. If

sand or sandy soil is wet and excessive pumping is encountered during compaction, the sand may be subcut 300 mm or more and replaced with a well graded, pit run material. A geotextile may be used as a separator at the base of the fill to reduce pumping of fines up into the fill, while allowing water to escape.

Structures can be supported on augered cast-in-place concrete piles designed on the basis of skin friction. However, standup conditions in sand will not be good and sleeving will likely be necessary to keep excavations open for concreting. Settlement of piles is expected to be less than 5 mm for a properly designed and constructed pile foundation. The skin friction contribution of the upper 2.0 m of pile below finished grade should be ignored in the determination of pile capacity for perimeter piles supporting a grade beam. This can be reduced to 1.0 m for interior piles.

An allowable skin friction value of 25 kPa may be used for design of piles in clay. A value of 15 kPa may be used where sand is encountered. The minimum length of pile should be 5 m. Grade beams should be constructed with a minimum 100 mm void space so that any heaving of the subgrade soil does not exert an upward force on piles, which can result in separation of the grade beam from the pile and distortion of the structure.

For augered piles, concrete should be placed within 2 hrs of excavation to minimize softening of clay or silt which can reduce pile capacity, or excessive sloughing and squeezing of soil, which can result in necking of the pile. The aspect ratio of a pile, defined as the ratio between length and diameter, should not exceed about 30. This should ensure that good contact is maintained between the concrete and soil and that no voids are created.

The use of water to facilitate excavation of piles should be avoided, since this will result in softening of the soil in contact with the concrete, reducing pile capacity. Inspection during construction is suggested to ensure compliance with specifications.

Landscaping around the structures should consider potential effects on foundation performance. Plantings of trees and large shrubs immediately adjacent the foundation should be avoided. Grading around the building should ensure positive drainage. Care should be taken to ensure that downspouts divert water away from the foundation.

6.3 Floors

Floors placed on a medium to high plasticity clay subgrade will experience some vertical movement associated with heave or shrinkage due to changes in soil moisture. The presence

of fill material of unknown or variable type and consistency may result in differential settlement of a grade supported floor. It is estimated that as much as 150 mm of heave may occur for floors constructed on medium to high plasticity clay. Little vertical movement is expected for a well constructed floor constructed on a sand subgrade.

The specification for compaction of clay subgrade soil should specifically indicate that the water content should be at optimum to optimum + 2 percent, since clay compacted wet of optimum will have a lower potential for heave. This will not, however, eliminate the potential for heave.

6.4 Potential for Sulphate Attack

The water soluble sulphate content of soil was measured to be 0.01 percent by dry weight of soil in sand and 0.14 percent in clay. On this basis, the potential for sulphate attack will be moderate to severe for concrete in contact with clay. Sulphate resistant (Type 50 or HS) cement must be specified for all concrete in contact with clay soil. Recommendations regarding sulphate resistant cement may be found in CSA A23.1.

7.0 Closure

This report was prepared by Clifton Associates Ltd. for the use of Mr. Neil Ketilson and his agents for specific application to the proposed Casa Grande subdivision south of Saskatoon. The material in it reflects Clifton Associates Ltd. best judgment available to it at the time of preparation. Any use which a third party makes of this report, or any reliance on or decisions to be made based on it, are the responsibility of such third parties. Clifton Associates Ltd. accepts no responsibility for damages, if any, suffered by any third party as a result of decisions made or actions based on this report.

This report has been prepared in accordance with generally accepted engineering practice common to the local area. No other warranty, expressed or implied is made

Our conclusions and recommendations are preliminary and based upon the information obtained from the referenced subsurface exploration. The boreholes and associated laboratory testing indicate subsurface and groundwater conditions only at the specific locations and times investigated, only to the depth penetrated and only for the soil properties tested. The subsurface conditions may vary between the boreholes and with time. The subsurface interpretation provided is a professional opinion of conditions and not a certification of the site conditions. The nature and extent of subsurface variation may not become evident until construction or further investigation. If variations or other latent conditions do become evident, Clifton Associates Ltd. should be notified immediately so that we may re-evaluate our conclusions and recommendations. Although subsurface conditions have been explored, we have not conducted analytical laboratory testing on samples obtained nor evaluated the site with respect to the potential presence of contaminated soil or groundwater conditions.

The enclosed report contains the results of our investigations as well as certain recommendations arising out of such investigations. Our recommendations do not constitute a design, in whole or in part, of any of the elements of the proposed work. Incorporation of any or all of our recommendations into the design of any such element does not constitute us as designers or co-designers of such elements, nor does it mean that such design is appropriate in geotechnical terms. The designers of such elements must consider the appropriateness of our recommendations in the light of all design criteria known to them, many of which may not be known to us. Our mandate has been to investigate and recommend which we have completed by means of this report. We have had no mandate to design, or review the design of, any elements of the proposed work and accept no responsibility for such design or design review.

Association of Professional Engineers and Geoscientists of Saskatchewan Certificate of Authorization No. 238





Symbols and Terms

Soil Descriptive Terms

A soil description for geotechnical applications includes a description of the following properties:

- texture
- color, oxidation
- consistency and condition
- primary and secondary structure

Texture

The soil texture refers to the size, size distribution and shape of the individual soil particles which comprise the soil. The Unified Soil Classification System (ASTM D2487-00) is a quantitative method of describing the soil texture. The basis of this system is presented overleaf. The following terms are commonly used to describe the soil texture.

-	article Size	Relativ	ve Proportions
	IM D2487-00)	(CFEM,	3rd Ed., 1992)
Boulder	300 mm plus	Trace	1 - 10 %
Cobble	75 - 300 mm	Some	10 - 20 %
Gravel	4.75 - 75 mm	Gravelly, sandy,	20 - 35 %
Coarse	19 - 75 mm	silty, clayey,	
Fine	4.75 - 19 mm	etc.	
Sand Coarse Medium	0.075 - 4.75 mm 2 - 4.75 mm 0.425 - 2 mm	And	>35 %
Fine	0.075 - 0.425 mm	Gravel, Sand,	>35 % and main fraction
Silt and Clay	Smaller than 0.075 mm	Silt, Clay	

Gradation

Particle Shape

Well Graded	Having a wide range of	Angular	Sharp edges and relatively
	grain sizes and substantial amount of all	, ungular	plane sides with unpolished surfaces.
Uniform or	intermediate sizes. Possessing particles of	Subangular	Similar to 'angular' but have rounded edges.
Poorly Graded Gap Graded	predominantly one size. Possessing particles of two distinct sizes.	Subrounded	Well-rounded corners and edges, nearly plane sides.
		Rounded	No edges and smoothly curved sides.
		Also may be	e flat, elongated or both.

The term "TILL" may be used as a textural term to describe a soil which has been deposited by glaciers and contains an unsorted, wide range of particle sizes.

Color And Oxidation

The soil color at its natural moisture content is described by common colors and, quantitatively, in terms of the Munsell color notation; (eg. 5Y 3/1). The notation combines three variables, hue, value and chroma to describe the soil color. The hue indicates its relation to red, yellow, green, blue and purple. The value indicates its lightness. The chroma indicates its strength of departure from a neutral of the same lightness.

Departure of the soil color from a neutral color indicates the soil has been oxidized. Oxidation of a soil occurs in a oxygen rich environment where most commonly metallic iron, oxidizes and turns a neutral colored soil 'rusty' or reddish brown. Oxidized manganese gives a purplish tinge to the soil. Oxidation may occur throughout the entire soil mass or on fracture/joint/fissure surfaces.

							ineering Purpos			
Major divisions Group Symbols Typical names			Typical names			Classification criter	ia			
	raction .75 mm)		GW	Well-graded gravel	oup name		$C_u = \frac{D_{60}}{D_{10}} \ge 4;$ $C_c = \frac{1}{10}$	$\frac{(D_{30})^2}{D_{10} X D_{60}}$ between 1 and 3		
mm)	mm) s coarse f sieve(≥4	s coarse f sieve(≥4.	s ⁺coarse f sieve(≥4	Clean gravels <5% fines	GP	Poorly graded grave	sand" to gro	ons Sumbole	Not meeting either C _u or	C _c criteria for GW
* (>0.075	Gravels More than 50% of coarse fraction retained on No. 4 sieve(≥4.75 mm)	Gravels with fines >12% fines	GM	Silty gravel	lf ≥ 15% sand add "with sand" to group name	on basis of percentage of fines No. 200 sieveGW, GP, SW, SP ss No. 200 sieveGM, GC, SM, SC 200 sieveborderline classifications	Atterberg limits below "A" line or PI less than 4	Atterberg limits plotting in hatched area are borderline classifications requiring use of dual symbols		
ed soils 200 sieve	More th retainec	Gravels with fi >12% fines	GC	Clayey gravel	lf ≥ 15% sa	centage o GW, G GM, G orderline o	Atterberg limits on or above "A" line and PI > 7	If fines are organic add "with orgnic fines" to group name		
rse-graine ed on No.	ion mm)	sands ines	SW	Well-graded sand	roup name	sis of per 30 sieve 200 sieve eveB		$\frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3		
Coa % retaine	Sands 9 of coarse fraction 4 sieve(<4.75 mm)	Sands coarse fracti eve(<4.75 n	Clean sands <5% fines	SP	Poorly graded sand	gravel to g	on on ba ss No. 20 ass No. o. 200 si	Not meeting either C _u or	C _c criteria for SW	
Coarse-grained soils More than 50% retained on No. 200 sieve* (>0.075 mm)	Sai Nore of coa No. 4 sieve	ith fines ines	SM	Silty sand	gravel add "with gravel to group name	Classification on basis of percentage of fines Less than 5% pass No. 200 sieveGW, GP, SW, SP More than 12% pass No. 200 sieveBorderline classificatio 5 to 12% pass No. 200 sieveBorderline classificatio	Atterberg limits below "A" line or PI less than 4	Atterberg limits plotting in hatched area are borderline classifications requiring use of dual symbols		
Moi	50% or more passes No. ²	Sands with fines >12% fines	SC	Clayey sand	lf≥15% gra	Less th More th 5 to 12	Atterberg limits on or above "A" line and PI > 7	If fines are organic add "with orgnic fines" to group name		
(mr	(E " %		s %	ji	ML	Silt	opriate iid limit		Plasticity Char	
e* (≤0.075 mm)	Silts and Clays iquid limit <50%	Inorganic	CL	Lean Clay -low plasticity	gravel" as appropriate s appropriate of undried liquid limit	Ec	i LL=16 to PI=7, then PI=0.9(LL- puation of A-Line: Horizontal PI=4 to 25.5, then PI=0.73(LL-2			
Fine-grained soils sses No. 200 sieve	Silts a Liquid	Organic	OL	Organic clay or silt (Clay plots above 'A' Line)	sand" or "with or "gravelly" as d limit is < 75%	40	3	10 to		
Fine-gr asses No	ays 50%	nic	ΜН	Elastic silt	d, add "with Idd "sandy" n dried liqui	sticity	U' Line	'A' Line		
Fine-grained soils 50% or more passes No. 200 sieve*	Silts and Clays Liquid limit ≥50%	Inorganic	СН	Fat Clay -high plasticity	If 15 to 29% coarse-grained, add "with sand" or "with gravel" as ap If > 30% coarse-grained , add "sandy" or "gravelly" as appropriate Class as organic when oven dried liquid limit is < 75% of undried li	10		OH or MH		
50		Organic	ОН	Organic clay or silt (Clay plots above 'A' Line)	If 15 to 29% If > 30% cc Class as or		10 20 30 40 50 6	0 70 80 90 100		
	Highly organic	soils	PT	Peat, muck and other highly organic soils			16 Liquid Limit (
*Based	on the mat	erial pass	sing the 3 in.	(75 mm) sieve, if field samples	contain co	bbles or boulder	s, add "with cobbles or boulders	" to group name		

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Consistency And Condition

The consistency of a cohesive soil is a qualitative description of its resistance to deformation and can be correlated with the undrained shear strength of the soil. The condition of a coarse grained soil qualitatively describes the soil compactness and can be correlated with the standard penetration resistance (ASTM D1586-99).

Consistency Of Cohesive Soil (CFEM, 3rd Edit., 1992)

Consistency	Undrained Shear Strength (kPa) (CFEM, 3rd Edt., 1992)	Field Identification (ASTM D 2488-00)
Very Soft Soft	<12 12-25	Thumb will penetrate soil more than 25 mm
Firm	25-50	Thumb will penetrate soil about 25 mm. Thumb will indent soil about 6 mm.
Stiff	50-100	Thumb will indent, but penetrate only with great effort (CFEM).
Very Stiff	100-200	Readily indented by thumbnail (CFEM).
Hard	>200	Thumb will not indent soil but readily indented with thumbnail.
Very Hard	N/A	Thumbnail will not indent soil.

Condition Of Coarse Grained Soil (CFEM, 3rd Edt., 1992)

Compactness Condition	SPT N - Index (Blows/300mm)
Very Loose	0 - 4
Loose	4 - 10
Compact	10 - 30
Dense	30 - 50
Very Dense	over 50

Moisture Conditions (ASTM D2488-00)

Description	Criteria
Dry	Absence of moisture, dusty, dry to touch
Moist	Damp but no visible water
Wet	Visible, free water, usually soil is below water table

Structure

The soil structure is the manner in which the individual soil particles are assembled to form the soil mass. The primary soil structure is the arrangement of soil particles as originally deposited. The secondary soil structure refers to any rearrangement of the soil such as deformation and cracking which has taken place since deposition.

Primary Soil Structure (Depositional)

A. Geometry		
Stratum	-	A single sedimentary 'layer', greater than 10 mm in thickness, visibly separable from other strata by a discrete change in lithology and/or sharp physical break.
Homogeneous	-	Same color and appearance throughout.
Stratified	-	Consisting of a sequence of layers which are generally of contrasting texture or color.
Laminated	-	Stratified with layer thicknesses between 2 mm and 10 mm.
Thinly laminated	-	Stratified with layer thickness less than 2 mm.
Bedded	-	Stratified with layer thicknesses greater than 10 mm.
Very Thinly Bedded (Flaggy)	-	Stratified with layer thicknesses between 10 and 50 mm.
Thinly Bedded (Slabby)	-	Stratified with layer thicknesses between 50 and 600 mm.
Thickly Bedded (Blocky)	-	Stratified with layer thicknesses between 600 and 1200 mm.
Thick-Bedded (Massive)	-	Stratified with layer thicknesses greater than 1200 mm.
Lensed	-	Inclusions of small pockets of different soils, such as small lenses of sand material throughout a mass of clay.
B. Bedding Structures	5	
Cross-bedding	-	Internal 'bedding' inclined to the general bedding plane.
Ripple-bedding	-	Internal 'wavy bedding'.
Graded-bedding	-	Internal gradation of grain size from coarse at base to finer at top of bed.

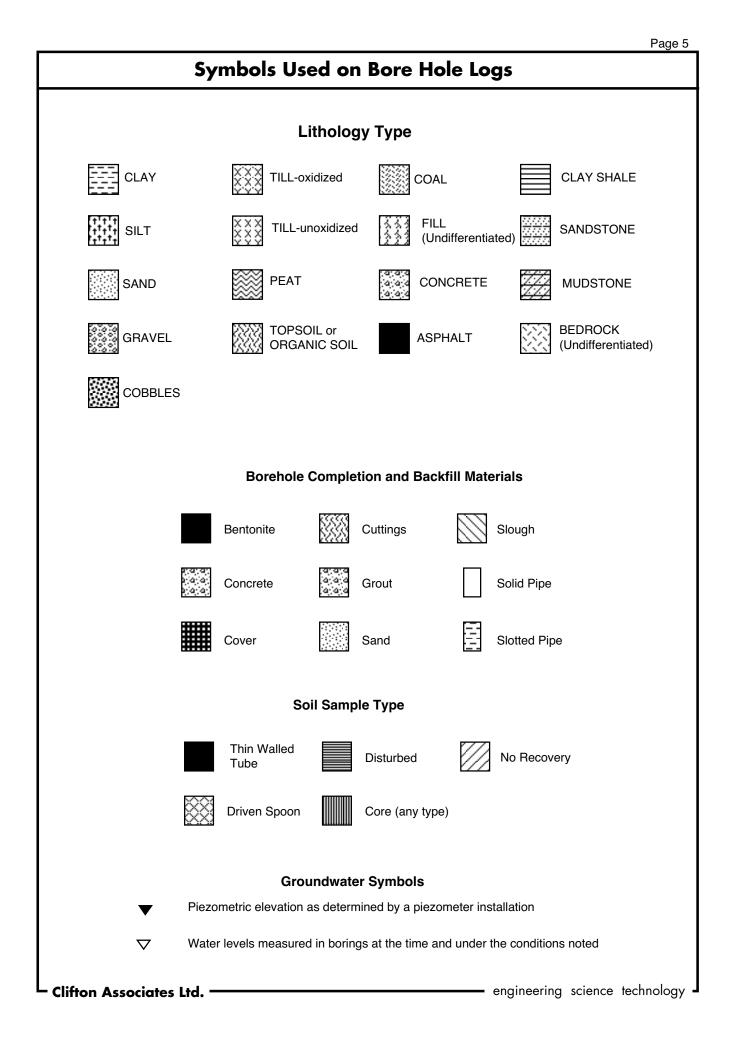
Horizontal bedded - Internal bedding is parallel and flat lying

Secondary Soil Structure (Post-Depositional)

A. Accretionary Structures

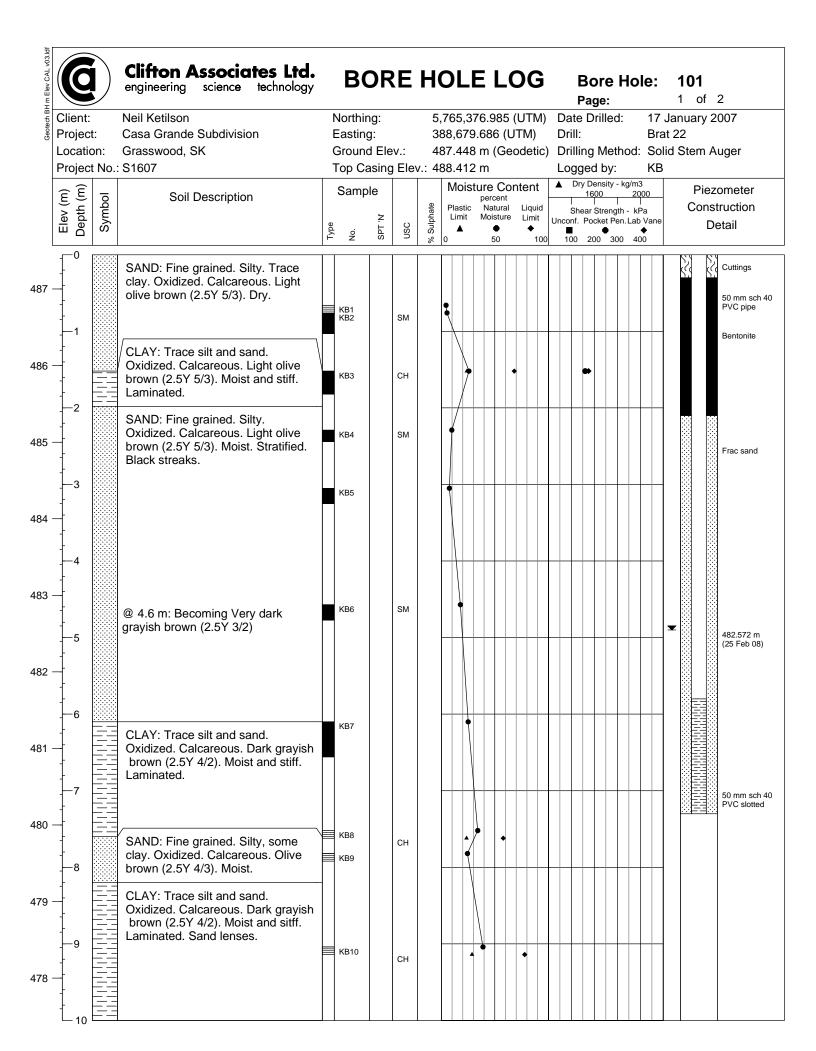
Includes nodules, concretions, crystal aggregates, veinlets, color banding and

Cementation	- Chemically precipitated material, commonly calcite (CaCO ₃), binds the grains of soil, usually sandstone. Described as weak, moderate, strong (ASTM D2488-00).
Salt Crystals	 Groundwater flowing through the soil/rock often precipitates visible amounts of salts. Calcite (CaCO₃), glauber salts (Na₂Ca(SO₄)₂), and gypsum (CaSO₄*2H₂O) are common.
B. Fracture	Structures
Fracture	 A break or discontinuity in the soil or rock mass caused by stress exceeding the materials strength.
Joint	 A fracture along which no displacement has occurred.
Fissure	 A gapped fracture, which may open and close seasonally. Usually an extensive network of closely spaced fractures, giving the soil a 'nuggetty' structure.
Slickensides	 Fractures in a clay that are slick and glossy in appearance, caused by shear movements.
Brecciated	 Contains randomly oriented angular fragments in a finer mass, usually associated with shear displacements in soils.
Fault	- A fracture or fracture zone along which there has been displacement.
Blocky	- A cohesive soil that can be broken down into small angular lumps which resist further breakdown.





Bore Hole Logs and Laboratory Test Data



Geotech BH m Elev CAL v03.ld1	G		Clifton Associates Ltd. engineering science technology		B	OR	RE	H	0	LE	ΞI	LC)G	Ì		Bo Pag		Ho	ole:		101 2 of	2
Geotech BH	Client: Project: Locatior Project	n:	Neil Ketilson Casa Grande Subdivision Grasswood, SK S1607		Northir Easting Ground Top Ca	g: d Ele		38 48	88, 87.	679. 448	686 m (85 (U 6 (U Geo	TM)	c)	Dat Drill Drill	e Di :	rille Me	thoc	Br	' Jai at 2 olid 3	nuary 2	007
		Symbol	Soil Description	Type	Samp		USC	% Sulphate	Pla Li	astic	perce Natu Mois 50	ural sture	Liqui Limit	d	SI nconf	160 near S . Poc	00 Streng ket P	 gth - I	000 kPa b Vano ∳	e	Cons	ometer truction etail
-777 -776 -775 -774 -773 -772 -771 -770 -699 -688	- 11 - 12 - 12 - 13 - 13 - 14 14 		CLAY: Trace silt and sand. Oxidized. Calcareous. Dark grayish brown (2.5Y 4/2). Moist and sitff. Laminated. Sand lenses. SILT: Sandy, trace clay. Oxidized. Calcareous. Very dark grayish brown (2.5Y 3/2). Moist and stiff. Dilatant. NOTES: 125 mm continuous flight auger used. Seepage @ 4.6 m.		KB11		СН															

BH m Elev CAL v03.ld	6		Clifton Associates Ltd. engineering science technology		B	DF	RE	H	0	LE	L	.00	6		ore	Hc	ole:	10 2 1	2 of 1	
Geotech Br	-	t: on:	Neil Ketilson Casa Grande Subdivision Grasswood, SK : S1607	 (Northir Easting Ground Top Ca	g: d Ele		3 4	88, 90.	,709.: .021 i	567 (m (G	0 (UTN (UTM) eodeti	ic)	Date Drill: Drillin Logge	Drille g Me ed by	ethod /:	Bra : So KB	at 22 Iid Ste	ry 2007 m Auger	
	Elev (m) Depth (m)	Symbol	Soil Description	Type	Samp	N. Lds	nsc	% Sulphate	PI	astic	re C Dercen Natura Moistur 50	al Liqui re Limi I	id	1	600 I r Strer ocket	 Igthk Pen.La	000 .Pa b Vane ∳	c	Piezometer onstruction Detail	
490	0 1 1		SAND: Fine grained. Silty, trace clay. Oxidized. Calcareous. Light olive brown (2.5Y 5/3). Dry and compact. Homogeneous.		KB13 KB14	14			•											
488	- - - 2				KB15 KB16	13			•											
				X		17														
487	3 				KB19 KB20	16														
486	4 4				KB21 KB22	18														
485	- 5 		CLAY: Trace sand and silt. Oxidized. Calcareous. Light olive brown (2.5Y 5/4). Moist and very stiff. Laminated. Iron staining. Silt partings. Lensed with sand.		NDZZ	10														
484	6 6 				KB23 KB24					•										
483	- 7 -		NOTES: 125 mm continuous flight auger used. No sloughing.																	
482	8 8 																			
481	- 9 -																			

		Clifton Associates Ltd. engineering science technology)G			Pag	je:	Но			103 1 of	
Client: Project Location Project	rt: on:	Neil Ketilson Casa Grande Subdivision Grasswood, SK S1607		Northii Eastin Groun Top Ca	g: d Ele		3 4	88,	747	.079	9 (U	UTM FM) detic	, (;	Drill Drill		Me	thoc	Br	at 2 blid	nuary 2 2 Stem A	
Elev (m) Depth (m)	Symbol	Soil Description	Type	Samp	ole N. Lds	USC	% Sulphate	Pl L	loist astic imit ▲	Ure perce Nate Mois	ural ture	Liquid Limit	Ur	Sł nconf.	160 l near S Poc	00 Stren ket F	 gth - I	1000 √Pa b Vane	e	Cons	ometer truction etail
		SAND: Fine grained. Some silt and clay. Oxidized. Calcareous. Light olive brown (2.5Y 5/3). Moist. Homogeneous.		KB25				•													
- - - -	+ + + + + + + + + + + + + + + + + +	SILT: Some clay. Oxidized. Calcareous. Light olive brown (2.5Y 5/4). Moist. Homogeneous.	_	KB27					•										-		
- - - 3	+ + + + + + + + + + + + + + +	SAND: Fine grained. Silty, some clay. Oxidized. Calcareous. Light olive brown (2.5Y 5/4). Moist. Homogeneous.		KB28				•											-		
- - - - - - - - - - - - - - - - - - -	+ + + + + + + + + + + + + + + + + + +	SILT: Some clay, trace sand. Oxidized. Calcareous. Light olive brown (2.5Y 5/4). Moist. Homogenous. Iron staining.	\																_		
- - 5	+++++++++++++++++++++++++++++++++++++++	clay. Oxidized. Calcareous. Light olive brown (2.5Y 5/4). Moist. Homogeneous. SILT: Some clay, trace sand.		KB29															-		
	+ + + + + + + + + + + + + + + + + + +	Oxidized. Calcareous. Light olive brown (2.5Y 5/4). Moist. Homogenous.		КВ30				•													
		SAND: Fine grained. Silty, some clay. Oxidized. Calcareous. Light olive brown (2.5Y 5/4). Moist. Homogeneous.																			
-7 		NOTES: 125 mm continuous flight auger used. No sloughing.																			
- 																					
- - - - -																					

BH m Elev CAL v03.ld	6		Clifton Associates Ltd. engineering science technology		BC	DR	RE	H	OL	_E	Ľ	OG	ì	Bo Pag		Hole	e:	104 1 of	[:] 1	
Geotech	Client: Project Location Project	t: on:	Neil Ketilson Casa Grande Subdivision Grasswood, SK S1607		Northir Easting Ground Top Ca	g: d Ele		3 4	89,0 89.0	77.5 12 r	547 (n (G	5 (UTM (UTM) eodetic	Dr Dr	ate D ill:	rilled Meth	nod:	Brat	anuary 22		
-	Elev (m) Depth (m)	Symbol	Soil Description	Type	Samp		usc	% Sulphate	Plas Lim 0	r stic nit N	re C bercen Natura Moistur 50	al Liquid	Unco	nf. Poc	00 Strengt ket Pe	kg/m3 2000 I h - kPa n.Lab V ∳ 0 400	ane	Cor	ezometer Instructior Detail	
489 - 488 -	0 1 		SAND: Fine grained. Silty, some clay. Oxidized. Calcareous. Light olive brown (2.5Y 5/3). Moist. Homogenous.		KB31		SM		•									ૻૻઽૻૼઽૻઽૻૼઽૻ૱	50 mm s PVC pip	
487 -	- - 2 -		CLAY: Silty, sandy. Oxidized. Calcareous. Light olive brown (2.5Y 5/3). Moist. Hard. Homogeneous.		КВ32 КВ33		CL		•		•							~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	5355355355	
486 -	3 		@ 3.04m: Trace sand. Iron staining. Silt partings.		KB34 KB35		СН					•			++•	+			Bentonit	e
484 -	- - - - - - - - -		SAND: Fine grained. Silty. Oxidized. Calcareous. Light olive brown (2.5Y 5/3). Moist. Iron and Manganese staining.		KB36 KB37		SM		•						++•	+			Frac sar	nd
483 -	6 6 		@ 6.1m: Clayey.		KB38															
482 -	- - - - - -				KB39		SM			•									482.262 (19 Marc 50 mm s PVC slo	ch 08) sch 40
481 -	8 9				KB40															
	- - - - -		NOTES: 125 mm continuous flight auger used. Sloughing @ 5.8 m. Seepage @ 5.8 m.		1 1040															

Client: Projec Locati Projec	:t: on:	Neil Ketilson Casa Grande Subdivision Grasswood, SK S1607		Northi Eastin Groun Top C	g: d Ele		38 48	89,0)77.	457	(UT	JTM) M) detic)	Dr Dr Lo	ate [ill: illing gge	d by	thoc :	Br I: So Kl	at 2 blid \$	nuary 2 2 Stem A	
Elev (m) Depth (m)	Symbol	Soil Description	Type	Samp 	ole N. Lds	nsc	% Sulphate	Pla: Lin	stic nit	Dre (perce Natu Moist 50	nt ral ure	Eiquid Liquid Limit 100	Unco	1 Shea nf. P	Density 600 r Streng Docket P 200 3	gth - I Pen. La	2000 I kPa ib Van	e	Const	ometer truction etail
0 1 1 		CLAY: 300 mm organic material. Silty. Oxidized. Calcareous. Light olive brown (2.5Y 5/6). Moist. Homogeneous. SAND: Fine Grained. Silty, some		KB41				•										_		
- - - - - - - - - - - - - - - - - - -		clay. Oxidized. Calcareous. Light olive brown (2.5Y 5/4). Moist. Stratified.		KB42 KB43				•										_		
- 3 				KB44			-													
- 4 				KB45					ł									_		
5 6				KB46														_		
- - - - - 7		NOTES: 125 mm continuous flight auger used. Sloughing @ 4.3 m. Seepage @ 4.3 m.		ND40			-													
- - - - - - 8																				
- - - - - - 9																				

Geotech BH m Elev CAL v03.ld	6		Clifton Associates Ltd. engineering science technology		B	OR	RE	H	01	LE	L	OG			3or Page		ole:		106 of 1	
Geotech Bł		t: on: t No.	Neil Ketilson Casa Grande Subdivision Grasswood, SK : S1607	E	Northir Easting Ground Fop Ca	g: d Ele		3 4	89,0 89.1)53.9 15 r	995 (m (G	9 (UTM) UTM) eodetic	ם ם (: ב	Date Drill: Drillin Logg	Dril ng M jed k	led: lethc by:	Br od: Sc KE	at 22 olid S	iuary 20 2 Stem Au	
	Elev (m) Depth (m)	Symbol	Soil Description	Type	Samp g	ole N_IAS	nsc	% Sulphate	Pla: Lin 0	F stic nit M	re Co percent Natura Moistur 50	al Liquid	Un	She She	1600 ear Str Pocke	ength - ength - t Pen.l	2000 kPa Lab Vane	•	Piezo Constr Det	uction
489			SAND: Fine grained. Silty. Oxidized. Calcareous. Light olive brown (2.5Y 5/4). Moist. Homogeneous.		KB47				•									_		
487 ·	- - - - - - - -		CLAY: Silty, sandy. Oxidized.		KB48 KB49					•								-		
486			Calcareous. Light olive brown (2.5Y 5/4). Moist. Hard. Laminated.		KB50					•								_		
485 -	- - - - - - - - - - - - - - - - - - -				KB51					•								_		
484	- - - - - - - - - - - - - - - - - - -		@ 4.6m: Trace sand. Iron and Manganese staining. Some calcium carbonate concretions.															_		
483			NOTES: 125 mm continuous flight		KB52 KB53									•				-		
482			auger used.																	
481 ·	- - - - - - - - - - - - - - - - - - -																	-		
480	- - - - - - - - - - - - - - - - - - -																	-		

Geotech BH m Elev CAL v03.ld	0		Clifton Associates Ltd. engineering science technology		B	DR	RE	H	O	LE	Ξ	LC	C			Bo Pag		Hc	ole:		107 1 of	1
L	lient: rojec ocatio rojec	t: on:	Neil Ketilson Casa Grande Subdivision Grasswood, SK : S1607		Northir Easting Ground Top Ca	g: d Ele		3 4	39,0	086	.44	6 (U	(UTM) TM) odetic	, ;)	Date Drill: Drilli Loge	: ing l	Met	hod	В	at 2	nuary 2 2 Stem A	
	Depth (m)	Symbol	Soil Description	Type	Samp		USC	% Sulphate	Pla Lii		pero Na Moi		Liquid Limit • 10	Ur	nconf.	<u>160</u> ear S) treng tet Pe	th - k en.La	0000 kPa bVan	e	Const	ometer truction etail
	—0 		SAND: Fine grained. Silty. Oxidized. Calcareous. Light olive brown (2.5Y 5/3). Moist. Homogeneous. @ 1.5m: Wet.		KB54 KB55 KB56				•											_		
	- - 				KB57					•										_		
	4 		CLAY: Silty, sandy. Oxidized. Calcareous. Dark grayish brown (2.5Y 4/2). Moist. Soft. Laminated. Manganese staining.		KB58						,									_		
	- 6 -		CLAY: Some silt. Oxidized. Calcareous. Very dark grayish brown (2.5Y 3/2). Moist. Stiff. Laminated. Iron staining. NOTES: 125 mm continuous flight		KB59						•									-		
	- 7 - -		auger used. Sloughing @ 2.1 m.																			
- - - - -	8 9																			-		
-	- - 																					

Geotech BH m Elev CAL v03.ld	C		Clifton Associates Ltd. engineering science technology		BC	DF	RE	HOLE LOG Bore Hole: 108 Page: 1 of 2
		on: on: ot No.	Neil Ketilson Casa Grande Subdivision Grasswood, SK : S1607	l		g: d Ele asing		5,765,324.170 (UTM) Date Drilled: 17 January 2007 389,461.073 (UTM) Drill: Brat 22 486.440 m (Geodetic) Drilling Method: Solid Stem Auger ev.: 487.390 m Logged by: KB
	Elev (m) Depth (m)	Symbol	Soil Description	Type	Samp	ole N. LAS	USC	Big Moisture Content percent Dry Density - kg/m3 1600 Piezometer Plastic Natural Limit Liquid Moisture Liquid Limit Shear Strength - kPa Unconf. Pocket Pen. Lab Vane Construction % 0 50 100 100 2000 Detail
486 - 485 -			SAND: Fine grained. Silty, some clay. Oxidized. Calcareous. Olive brown (2.5Y 4/3). Moist. Compact. Homogeneous.		KB60 KB61 KB62	11	SM	So mm sch 40 PVC pipe Cuttings Bentonite
484 - 483 -	- - - - - - - - - - - - - - - - - - -		CLAY: Silty. Oxidized. Calcareous. Light olive brown (2.5Y 5/4). Moist. Soft. Iron staining. Laminated. Lensed with sand. SAND: Fine grained. Silty, some clay. Oxidized. Calcareous. Olive brown (2.5Y 4/3). Moist. Compact. Manganese staining.		KB63 KB64 KB64B KB65 KB66	9	SM	Frac Sand
482 -	4 		Homogeneous.		KB67			▲ 482.400 m (25 Feb 08)
480 -	- - - - - - - - - - - - - - - - - - -		CLAY: Trace silt. Oxidized. Calcareous. Very dark grayish brown (2.5Y 3/2). Moist. Firm. Laminated.		KB68 KB69		сн	
479 -	8		SAND: Fine grained. Silty. Oxidized. Calcareous. Dark grayish brown (2.5Y4/2). Wet. Homogeneous.		KB70			50 mm sch 40 PVC slotted
478 - 477 -	-7 -7 -7 -7 -7 -7 -7 -7 -7 -7 -7 -7 -7 -				KB71			

Geotech BH m Elev CAL v03.ld		0		Clifton Associates Ltd. engineering science technology		B	DR	E	H	0	LI	Ε	L	00	3			ore		lol	e:		108 2 of	2
Geotech BH	Clie Pro	ject atic	t: on:	Neil Ketilson Casa Grande Subdivision Grasswood, SK	 (Northin Easting Groun	g: d Ele		38 48	39,4 36.4	461 440	.07 m	3 (U	(UTN JTM) odet		Dr Dr	te E II: Iliną	Drille g Me	ed: etho	od:	Bra So	Jar at 22 lid S	nuary 2	007
		Depth (m)	Symbol	Soil Description	1	Top Ca Samp		SU	Sulphate	M Pla Li		pero Na Moi	Cont tural sture	nten Liqu Limi	id it	Unco	Sheai hf. Po	Densit 600 Stre Docket	ty - kg I ngth Pen.	g/m3 200 - kPa Lab \	a /ane		Const	ometer ruction etail
476 475 474 473		 10 11 12 13		SAND: Fine grained. Silty. Oxidized. Calcareous. Dark grayish brown (2.5Y4/2). Wet. Homogeneous. NOTES: 125 mm continuous flight auger used. Water @ 6.9 m. Seepage @ 4.9 m.		KB72					•													
472		15																						
470		17																						
469		18																						
467		19 20																						

Geotech BH m Elev CAL v03.ld	Clifton Associates Ltd. engineering science technology				Page : 1 o												109 1 of	1				
L	Client: Project: Location: Project No		Neil Ketilson Casa Grande Subdivision Grasswood, SK o.: S1607		Northii Eastin Groun Top Ca	5,765,594.201 (UTM) 389,458.870 (UTM) 486.558 m (Geodetic) /.:							Date Drilled: 17 J Drill: Brat					rat 2 olid				
i	Elev (m) Depth (m)	Symbol	Soil Description	Type	Samp	ole N. LAS	usc	% Sulphate	Pla Li	lois astic imit	per Na Moi	Cont itural sture	ntent Liquid Limit • 10	Ur	S ncont	Dry De 160 hear \$ f. Poo) 20	00 Stren ket F	l gth - Pen.L	2000 I kPa ab Vai ♠	ne	Cons	ometer truction etail
36 —			SAND: Fine grained. Silty. Oxidized. Calcareous. Olive brown (2.5Y 4/3). Moist. Homogeneous.		KB74 KB75				•													
4 —			CLAY: Silty, sandy. Oxidized. Calcareous. Olive brown (2.5Y 4/3). Moist. Soft. Homogeneous. Iron and Manganese staining.		KB76 KB77 KB78				•													
2			 @ 4.3 m: Wet. SAND: Fine grained. Silty. Oxidized. Calcareous. Dark grayish brown (2.5Y 4/2). Moist. Iron staining. 		KB79 KB80					•				•								
	- 6 6 		NOTES: 125 mm continuous flight auger used. Sloughing @ 4.3 m.		KB81															_		
8 — 7 —	- - - - - - - - - - - - - - - - - - -																			_		

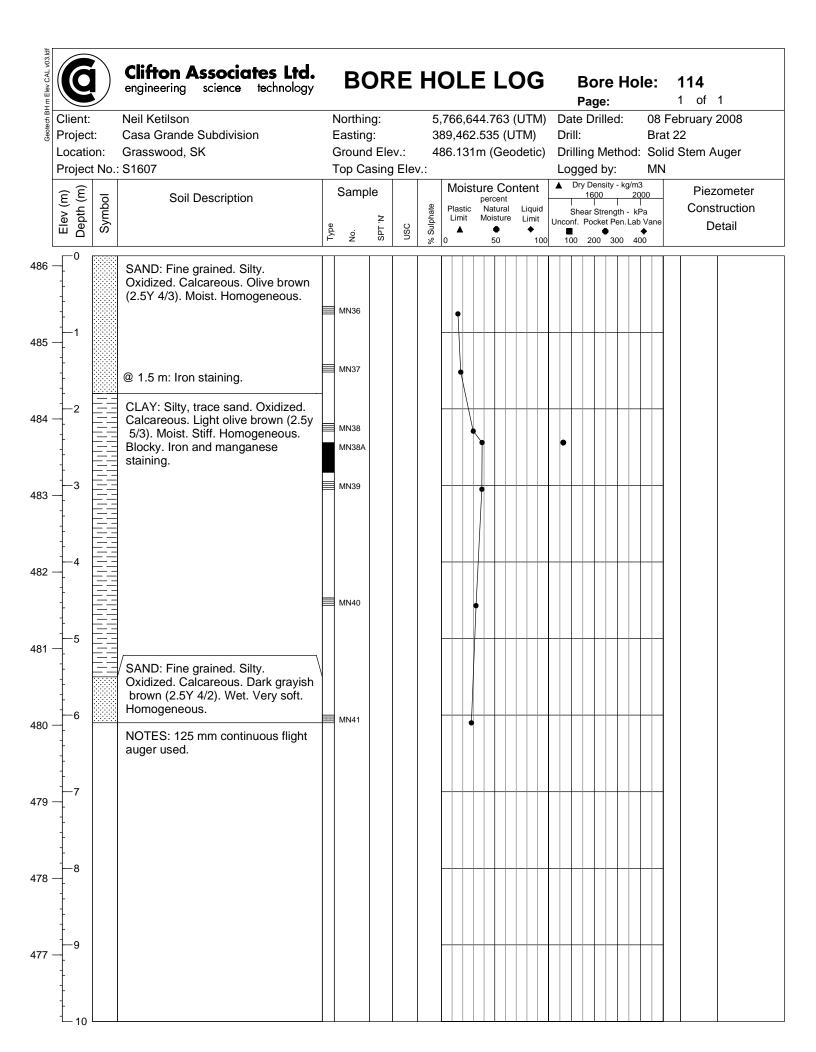
l m Elev CAL v03.ldf	engineering science technology Client: Neil Ketilson Project: Casa Grande Subdivision Location: Grasswood, SK Project No.: S1607		Clifton Associates Ltd. engineering science technology		BORE HOLE LOG									Bo Pag		Но	le:	110 1 of 1		
Geotech BH			Casa Grande Subdivision : Grasswood, SK		Northing: Easting: Ground Elev.: Top Casing Elev.			3 4	89,39 87.04	97.0 I3 m	72 (L 1 (Ge	odetic	Dr) Dr Lc	ate D ill: illing ggeo	rilleo Met d by:	hod:	Bra Sol KB	' January 2008 at 22 blid Stem Auger		
	Elev (m) Depth (m)	Symbol	Soil Description	Type	Samp	N ^I TAS	USC	% Sulphate	Moi Plasti Limit 0	pe c N	e Co ercent latural oisture 50	ntent Liquid Limit (0	Unco	l Shear nf. Po	00 Streng cket Po	th - kF	00 Pa Vane ♦	Cons	zometer struction Detail	
487	0 1		CLAY: Silty. Oxidized. Calcareous. Light olive brown (2.5Y 5/4). Moist. Homogeneous.		KB82				•											
			SAND: Silty. Oxidized. Calcareous. Dark olive brown (2.5Y 3/3). Moist. Homogeneous.		KB83				┥											
485	2		CLAY: With silt. Oxidized. Calcareous. Olive brown (2.5Y 4/3). Moist. Firm. Laminated. Iron staining.		KB84					•										
484	3		SAND: Fine grained. Silty, some		KB85				f											
483	4 4 		clay. Oxidized. Calcareous. Olive brown (2.5Y 4/3). Moist. Homogeneous.		KB86				•											
482																				
481			NOTES: 125 mm continuous flight auger used. Sloughing @ 4.3 m.		KB87					•										
479	-† 																			
478	9 10																			

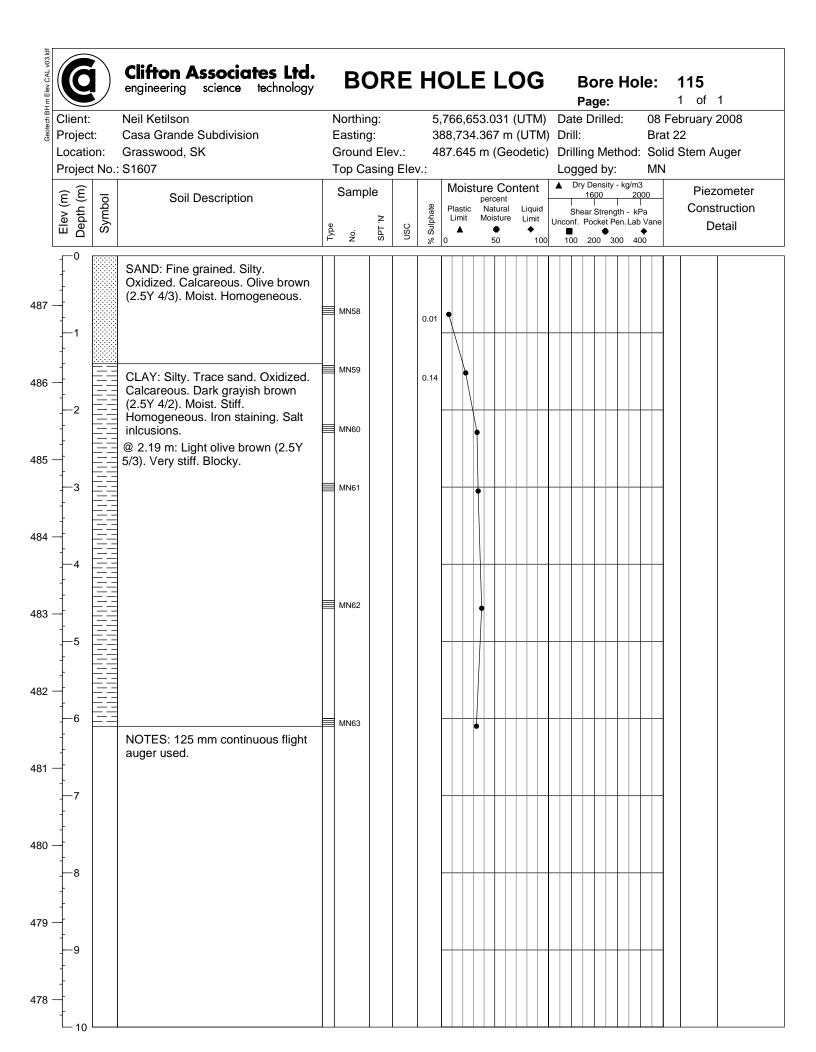
H m Elev CAL v03.k	Client: Neil Ketilson Project: Casa Grande Subdivision Location: Grasswood, SK Project No.: S1607			BORE HOLE						.OG		Bor Page	e Ho	ole:	e: 111 1 of 2			
			Casa Grande Subdivision Grasswood, SK			Northing: Easting: Ground Elev.: Top Casing Elev.				906.06 5.333 1 m (0 4 m	Dri Dri Dri	lling M gged b	lethod by:	Bra I: Sol MN	8 February 2008 Brat 22 Golid Stem Auger IN			
	Elev (m) Depth (m)	Symbol	Soil Description	Type	Samp g	ole N. Lds	nsc	% Sulphate		percer	ral Liquid	Uncor	1600 Shear Str f. Pocke	sity - kg/m 2 ength - k et Pen. La 300 4	000 I kPa b Vane ∳	Cor	ezometer hstruction Detail	
487 -			SAND: Some silt, some clay. Calcareous. Oxidized. Olive brown (2.5Y 4/3). Moist. Homogeneous.		MN25 MN26				•								50 mm sch 40 PVC pipe Bentonite	
485 -					MN27				•							▼	485.264 m (25 Feb 08) Frac sand	
484 -			CLAY: Some silt, some sand. Calcareous. Oxidized. Olive gray (5Y 4/2). Moist. Firm. Iron and manganese stains. Homogeneous.		MN28													
483 -	- 4 		SAND: And silt. Calcareous. Oxidized. Olive gray (5Y 4/2). Moist. Manganese stains. Homogeneous.		MN29													
482 -																	50 mm sch 40 PVC slotted	
481 -			CLAY: Some silt. Unoxidized. Calcareous. Dark gray (2.5Y 4/1). Moist. Stiff. Homogeneous. Blocky.		MN30					•								
480 -					MN31					•								
479 -																		
478 -	-9				MN32					•								

BH m Elev CAL v03.ldf		Clifton Associates Ltd. engineering science technology	В	OF	RE	H	OI	.E	LC)G				Hole		111 2 of	2
Geotech BH r	Client: Project: Location: Project No.	Neil Ketilson Casa Grande Subdivision Grasswood, SK : S1607	North Easti Grou Top (38 48	39,4 37.2 38.0	75.33 81 m 94 m		ΓM) detic)						February 2008 at 22 Ilid Stem Auger			
	Elev (m) Depth (m) Symbol	Soil Description	Sam	ple	nsc	% Sulphate	Plas Lim 0	pe tic N it Mo	e Con rcent atural bisture 50	tent Liquid Limit • 100	Uncor	<u>160</u> Shear S If. Pock	0 trength cet Per	<u>2000</u> n - kPa n.Lab V	ane	Const	ometer ruction etail
477		CLAY: Some silt. Unoxidized. Calcareous. Dark gray (2.5Y 4/1). Moist. Stiff. Homogeneous. Blocky.	<u></u> мN33					•									
476																	
475			MN34					•									
474	1 –	NOTES: 125 mm continuous flight	MN35					•									
473	1	auger used. Sloughing @ 4.3 m. Seepage @ 5.8 m.															
472	1																
471	1																
470																	
469																	
468	19 																

Clier Proje Loca Proje	ect atio	n:	Neil Ketilson Casa Grande Subdivision Grasswood, SK S1607		Northii Eastin Groun Top Ca	g: d Ele		38 48	39,0 37.1)67. 154	.950 m (0 (U (Geo	(UTM) TM) odetic	D ;) D L	ate rill: rilli ogg	ng I jed	lled Meth by:	nod:	Bra So Mi	at 2 olid 3	bruary 2 Stem A	
Elev (m)		Symbol	Soil Description	Tvpe	Samp	ole	USC	% Sulphate	Pla: Lin 0	stic nit	ure perc Nat Mois	ent tural sture	Liquid Limit	Unc	She conf.	1600 ear St Pock	isity -) trength et Per 300	20 n - kF n.Lab	00 Pa Vane	•	Cons	cometer struction etail
			SAND: Fine grained. Silty. Oxidized. Calcareous. Olive brown (2.5Y 4/4). Moist. Homogeneous.		MN13																	
			@ 1.5 m: And silt. Light olive brown		MN14					•												
1 1-2 1			(2.5Y 5/4). Salt inclusions. CLAY: Silty. Trace sand. Oxidized. Calcareous. Light olive brown		MN15															-		
- 3 3	-		(2.5Y 5/4). Homogeneous. Iron staining.		MN16															-		
- - 4	-		@ 4.3 m: Wet.																			
- - 5	-			_	MN17					•												
- - - 6	-		CLAY: Some silt. Unoxidized. Calcareous. Dark gray (2.5Y 4/1). Moist. Stiff. Homogeneous. Blocky.		MN18																	
			NOTES: 125 mm continuous flight auger used. Seepage @ 4.3 m.								,											
1-7 1 1																						
- 8 																				-		
- 9 								·														

Geotech BH m Elev CAL v03.ld	0		Clifton Associates Ltd. engineering science technology		BC	DR	Ε	H	Ol	_E	Ľ	.00	3		Bo Page		Ho	le:	113 1 of	1
Geotech Br	-	t: on: t No.:	Neil Ketilson Casa Grande Subdivision Grasswood, SK S1607	 (Northir Easting Ground Top Ca	g: d Ele		38 48	89,0 87.6	78.5 16 r	575 (m (G		ic)	Dat Dril Dril Log	e Dri I: ling I ged	lled Meth by:	nod:	Bra Sol MN		uger
	Elev (m) Depth (m)	Symbol	Soil Description	Type	Samp	le N. LAS	usc	% Sulphate	Plas Lim	r itic iit N	Natura Moistur 50	al Liqui re Limi ♦	id	l S Uncon	Dry Der 160(hear St f. Pock 0 200	rengt et Pe	20 h - kF n.Lab	00 Pa Vane ♦	Cons	ometer truction etail
487 - 486 -	0 		CLAY: Silty, sandy. Oxidized. Calcareous. Olive brown (2.5Y 4/3). Dry. Homogeneous. SAND: Fine grained. Oxidized. Calcareous. Olive brown (2.5Y 4/3). Dry. Homogeneous.		MN19 MN20				•											
485 - 484 -	- 2 		 @ 2.3 m: Moist. CLAY: And silt. Oxidized. Calcareous. Olive brown (2.5Y 4/3). Moist. Very soft. Homogeneous. Iron and manganese staining. Salt inclusions. 		MN21 MN22					•										
483 -	- 4 - - - 5 -		@ 3.7 m: Trace silt.		MN23						•									
482 -	- - - - - - - - -		CLAY: Trace silt. Unoxidized. Calcareous. Dark gray (2.5Y 4/1). Moist. Stiff. Homogeneous. Iron and manganese staining. NOTES: 125 mm continuous flight auger used.		MN24					•	•									
481 -																				
479 -	- 																			
478 ·	- - - - - - 10																			





Geotech BH m Elev CAL v03.ld	6		Clifton Associates Ltd. engineering science technology		BC	DR	RE	H	0	LE	EL	.00	3			ore ge:	e H	ole:		116 1 of	1
	Client: Projec Locatio	t: on:	Neil Ketilson Casa Grande Subdivision Grasswood, SK	E	lorthir Easting Ground	g: d Ele		3 4	89,	291.	604	8 (UTN (UTM) Geodet		Dri Dri	ite D ill: illing	orille Me	etho	Bi d: Si	rat 2 olid	bruary : 2 Stem A	
		t No.:	S1607	T	op Ca	asing	Elev	/.:							gge	-		Μ	N		
	Elev (m) Depth (m)	Symbol	Soil Description	Type	Samp g	sPT 'N' SPT 'N'	USC	% Sulphate	Pla Li	astic	percen Natura Moistu	al Liqu re Limi ♦	ıid it	Unco	16 Shear nf. Po	S00 Strer	l ngth - Pen.L	2000 I kPa ab Van	e	Cons	ometer ruction etail
L	— 0				z	0		%	0		50	,	100	10	00 2	00 :	300	400		1	
			SAND: Fine grained. Silty. Oxidized. Calcareous. Olive brown (2.5Y 4/3). Moist. Homogeneous. Iron staining. Salt inclusions.		MN1					•											
186 -	1 		CLAY: Silty. Oxidized. Calcareous. Dark olive brown (2.5Y 3/3). Moist. Stiff. Homogeneous. Iron staining. Salt inclusions.		MN2					•											
85 -	2				MN3																
84 -	- - 				MN4																
183 -	4 				MN5																
82 -	- 5 -		@ 5.2 m: Interbedded clay and silt.																		
81 -	- - 6				MN6																
			NOTES: 125 mm continuous flight auger used. Seepage @ 5.8 m.																		
80 -	- - 7 - -																		-		
79 -	- - 																		-		
	-																				
178 -	9 																				
- 77	- - - - 10																				

Client		Clifton Associates Ltd. engineering science technology		B	OR	E	H	Ol	.E	L	OG			ore	Ho	ole:		17	I
Locati Projec	ct: ion: ct No.	Neil Ketilson Casa Grande Subdivision Grasswood, SK : S1607	E	Northii Eastin Groun Fop Ca	g: d Ele		3 4	88,7 86.0	92.8 94 n	867 (L n (Ge	odetic	Di Di La	ogge	g Me d by	ethoc /:	Br I: Sc Mi	at 22 Iid S	tem Au	
Elev (m) Depth (m)	Symbol	Soil Description	Type	Samp g	ole N. Tas	nsc	% Sulphate	Plas Lim 0	p itic l it N	re Co ercent Natural Moisture 50	ntent Liquid Limit + 100	Unco	1 Shea onf. P	600 r Strer ocket	l Igth - I	2 <u>000</u> ↓ kPa ib Vane		Const	ometer ruction tail
		SAND: 900 mm organic material. Silty, trace clay. Oxidized. Calcareous. Olive brown (2.5Y 4/4). Moist. Homogeneous.		MN7 MN8				•									-		
		CLAY: Some silt. Oxidized. Calcareous. Olive brown (2.5Y 4/3). Moist. Homogeneous. Iron staining. Salt inclusions. Blocky.		MN9 MN10					•								-		
- - - - - - - - - - - - - - - - - - -				MN11													-		
		 4.6 m: Very dark grayish brown (2.5Y 3/2). 5.5 m: Sand lense. Wet. 																	
- 6 6		NOTES: 125 mm continuous flight auger used.		MN12															
- - - - - - - - - - - - - - - - - - -																			
9 9 9 																			

Clien			Clifton Associates Ltd. engineering science technology		B	DR	RE	H	0	LE	EL	.00)		Bo Pag		Н	ole	:	118 1 of	1
Loca Proje	ect: atio ect	n:	Neil Ketilson Casa Grande Subdivision Grasswood, SK S1607		Northir Eastin Groun Top Ca	g: d Ele		3 4	88,8 89.3	351.4 390 i	487 m (G	3 (UTM (UTM) Geodeti	ic)	Dri Dri Log	ll: lling ggeo	d by	tho :	E Id: S N	Brat	ebruary 22 Stem A	
Elev (m) Denth (m)		Symbol	Soil Description	Type	Samp		nsc	% Sulphate	Pla: Lir	stic	Percer Natur Moistu 50	al Liqui ire Limit	id	Uncon	16 Shear f. Po	00 Stren	 gth - Pen.L	2000 kPa .ab Va	ine	Cons	ometer truction etail
			SAND: Fine grained. Some silt. Oxidized. Calcareous. Light olive brown (2.5Y 5/4). Moist. Homogeneous.		MN52				•												
+-1 + + + +			CLAY: Sitly, trace sand. Oxidized. Calcareous. Olive brown (2.5Y 4/3). Moist. Very stiff. Homogeneous.		MN53																
2 			@ 2.2 m: Silt partings. Iron staining.		MN54																
	- - - - - - - - - - - - - - - - - - -		SAND: Fine grained. Silty. Oxidized. Calcareous. Dark grayish brown (2.5Y 4/2). Moist. Laminated.		MN55 MN55A				Ń												
- - - - 5			CLAY: And silt. Oxidized. Calcareous. Dark grayish brown (2.5Y 4/2). Moist. Very soft. Homogeneous. Iron staining. Organic inclusions.		MN56																
	· · · · · · ·		SAND: Silty. Oxidized. Calcareous. Olive brown (2.5Y 4/3). Wet. Homogeneous.		MN56																
+ -+ + + +-7			NOTES: 125 mm continuous flight auger used. Seepage @ 5.8 m.																		

Geotech BH m Elev CAL v03.ld	C		Clifton Associates Ltd. engineering science technology		B	OF	RE	H	0	LE	EI	LO	G			or		lol	le:		1 19 of	2
		t: on:	Neil Ketilson Casa Grande Subdivision Grasswood, SK : S1607	l (Northi Eastin Groun Top C	g: d Ele		3 4	89,: 88.:	262. 243	166 m (27 (U 6 (UTN Geod	/I) etic)	Di Di Lo	ogge	g M ed b	leth by:	od:	Bra Sol MN	at 22 lid S	oruary 2 2 Stem A	
	Elev (m) Depth (m)	Symbol	Soil Description	Type	Samp <u>g</u>	ole N. LAS	USC	% Sulphate	Pla Li	astic	perce Natu Moisi 50	ural Li ture L	ent quid imit ♦ 100	Unce	∫ Shea onf. F	ar Stre ocke	ength t Pen	kg/m3 200 1 - kP 1.Lab 40	00 'a Vane ∳		Const	ometer ruction etail
8 - 7 -			SAND: Fine grained. Silty. Oxidized. Calcareous. Light olive brown (2.5Y 5/6). Moist. Homogeneous.		MN42				•													
6 -	- - - - - - -				MN44 MN44A																	
; -					MN45																	
			CLAY: Silty. Oxidized. Calcareous.		MN46					•												
_	- - - - - - - - - - - - - - - - - - -		 Olive brown (2.5Y 4/3). Moist. Homogeneous. CLAY: Silty. Unoxidized. Calcareous. Very dark gray (2.5Y 3/1). Moist. Homogeneous. 		MN47					•												
_	- - - - - - - - -		SAND: Fine grained. Silty, trace clay. Unoxidized. Calcareous. Very dark gray (2.5Y 3/1). Wet. Homogeneous.		MN48					•												
_	- 																					
. –	9 				MN49																	

Client: Project:		Clifton Associates Ltd. engineering science technology		BC	DR	RE	H	0	LE		LO	G		Bc Pag		Но	le:		1 19	2
Location Project	n:	Neil Ketilson Casa Grande Subdivision Grasswood, SK S1607	E	Northir Easting Ground Fop Ca	g: d Ele		38 48	39,2 38.2	262. 243	166 m (M) etic)	Dri Dri Lo	lling ggeo	Met d by:	hod	Bra So MN	at 22 Iid S	oruary 2 2 Stem A	
	Symbol	Soil Description	Type	Samp		usc	% Sulphate	Pla Lir	stic	perce Natu Mois 50	ural L ture L	ent iquid .imit ♦ 100	Uncoi	Shear	00 Streng cket P	<u>2</u> th - k en.Lat	000 Pa o Vane		Const	ometer ruction etail
		SAND: Fine grained. Silty, trace clay. Unoxidized. Calcareous. Very dark gray (2.5Y 3/1). Wet. Homogeneous. @ 10.7 m: Clayey. Black (2.5Y 2.5/1).		MN50					•											
		NOTES: 125 mm continuous flight auger used. Seepage @ 6.4 m.		MN51					•											
- - - - - - - - - - -																				
- - - - - - - - - - - - - - - - - - -																				

		S	SUMN	/IARY	OF	SAM	PLIN	g an		ABOR	ATC	RY [·]	TEST	DAT	Ά		
	SAN	IPLE				CONSI	STENCY			GRAD	ATION			SHEAR	R STR	ENGTH	
DЕРТН	NUMBER	ТҮРЕ	RECOVERY	WATER CONTENT	PLASTIC LIMIT	LIQUID LIMIT	PLASTICITY INDEX	USC	GRAVEL	SAND	SILT	CLAY	SULPHATE CONTENT	COMPRESSION TEST	LAB VANE	POCKET PEN	DRY DENSITY
meters			mm	%	%	%	%		%	%	%	%	%	kPa	kPa	kPa	kg/m ³
0.76	KB1	BAG		4.4													
0.76	KB2	SY	270	5.3			NP	SM	0.0	84.2	16	6.8			N/A	N/A	
1.52	KB3	SY	300	25.8	23.9	68.2	44.3	СН	0.0	1.4	98	3.6			175	160	
2.29	KB4	SY	150	9.8			NP	SM	0.0	712.0	28	3.0			N/A	N/A	
3.05	KB5	SY	200	7.5											N/A	N/A	
4.57	KB6	SY	200	17.8			NP	SM	0.0	85.9	14	1.1			N/A	N/A	
6.10	KB7	SY	460	25.1													
7.62	KB8	BAG		34.0	23.6	57.9	34.3	СН	0.0	8.3	91	.7					
7.92	KB9	BAG		24.4													
9.14	KB10	BAG		38.9	28.7	77.9	49.2	СН	0.0	1.7	98	3.3					
10.67	KB11	BAG		34.6	24.0	74.6	50.6	СН	0.0	1.8	98	3.2					
12.19	KB12	BAG		27.0													
		Clifto enginee	n As: pring sc	social	t es Lt e echnolo	d. gy	PROJE LOCAT PROJE	ION	Grassw	irande Su vood, Sas					BOR	E HOL 101	E NO.

		S	UMN	/IARY	′ OF	SAM	PLIN	G AN	ND LA	BOF	RATO	RY 1	EST	DAT	Ά		
	SAN	IPLE				CONSI	STENCY			GRAD	ATION			SHEAF	R STR	ENGTH	
DEPTH	NUMBER	ТҮРЕ	RECOVERY	WATER CONTENT	PLASTIC LIMIT	LIQUID LIMIT	PLASTICITY INDEX	USC	GRAVEL	SAND	SILT	сгау	SULPHATE CONTENT	COMPRESSION TEST	LAB VANE	POCKET PEN	DRY DENSITY
meters			mm	%	%	%	%		%	%	%	%	%	kPa	kPa	kPa	kg/m ³
0.76	KB13	BAG		4.2					0.0	76.7	13.2	10.1					
0.76	KB14	SPT		3.8													
1.52	KB15	BAG		5.1													
2.29	KB16	SPT		4.9													
3.05	KB17	BAG		4.9													
4.57	KB18	SPT		4.7													
6.10	KB19	BAG		4.5													
7.62	KB20	SPT		4.2													
7.92	KB21	BAG		23.3													
9.14	KB22	SPT		26.2													
10.67	KB23	BAG		30.9													
12.19	KB24	SPT		27.5													
C				social sience t			PROJE LOCAT PROJE		Grassw	rande Si ood, Sas					BOR	E HOL 102	E NO.

		S	UMN	/IARY	′ OF	SAM	PLIN	g an	ID LA	BOF	RATO	RY 1	rest	DAT	Ά		
	SAN	IPLE				CONSIS	STENCY			GRAD	ATION			SHEAR	R STR	ENGTH	
ДЕРТН	NUMBER	ТҮРЕ	RECOVERY	WATER CONTENT	PLASTIC LIMIT	LIQUID	PLASTICITY INDEX	nsc	GRAVEL	SAND	SILT	CLAY	SULPHATE CONTENT	COMPRESSION TEST	LAB VANE	POCKET PEN	DRY DENSITY
meters			mm	%	%	%	%		%	%	%	%	%	kPa	kPa	kPa	kg/m ³
0.76	KB25	BAG		6.4													
1.52	KB26	BAG		6.4													
2.29	KB27	BAG		12.6													
3.05	KB28	BAG		4.2													
4.57	KB29	BAG		11.7													
6.10	KB30	BAG		6.9													
C		Clifto enginee					PROJE(LOCATI PROJE(ON	Grassw		ubdivisio skatchew				BOR	E HOL I 103	E NO.

		S	UMN	IARY	OF	SAM	PLIN	G AN	ND LA	BOR	ATC	RY	rest	DAT	Ά		
	SAM	PLE				CONSI	STENCY			GRAD	ATION			SHEAI	R STR	ENGTH	
DEPTH	NUMBER	ТҮРЕ	RECOVERY	WATER CONTENT	PLASTIC LIMIT	LIQUID LIMIT	PLASTICITY INDEX	nsc	GRAVEL	SAND	SILT	СГАУ	SULPHATE CONTENT	COMPRESSION TEST	LAB VANE	POCKET PEN	DRY DENSITY
meters			mm	%	%	%	%		%	%	%	%	%	kPa	kPa	kPa	kg/m ³
0.76	KB31	BAG		9.2			NP	SM	0.0	55.4	44	1.6					
1.52	KB32	BAG		11.8	19.5	43.3	23.8	CL	0.0	36.0	64	1.0					
2.29	KB33	BAG		16.5	18.9	49.1	30.2	CL	0.0	17.7	82	2.3					
3.05	KB34	BAG		18.6													
3.05	KB35	SY	310	23.4	26.4	63.7	37.3	СН	0.0	1.0	99	9.0			260+	290+	
4.57	KB36	BAG		13.5													
4.57	KB37	SY	260	9.1			NP	SM	0.0	57.0	43	3.0			260+	290+	
6.10	KB38	BAG		16.3													
7.62	KB39	BAG		28.7			NP	SM	0.0	77.3	22	2.7					
9.14	KB40	BAG		25.6													
C					tes Lt e echnolo		PROJE LOCAT PROJE		Grassw	rande Su ood, Sas					BOR	E HOL I 104	E NO.

		S	UMN	/IARY	′ OF	SAM	PLIN	g an	ID LA	BOR	RATC	RY 1	EST	DAT	Ά		
	SAN	IPLE				CONSIS	STENCY			GRAD	ATION			SHEAF	R STRI	ENGTH	
ДЕРТН	NUMBER	ТҮРЕ	RECOVERY	WATER CONTENT	PLASTIC LIMIT	LIMIT	PLASTICITY INDEX	nsc	GRAVEL	SAND	SILT	СГАҮ	SULPHATE CONTENT	COMPRESSION TEST	LAB VANE	POCKET PEN	DRY DENSITY
meters			mm	%	%	%	%		%	%	%	%	%	kPa	kPa	kPa	kg/m ³
0.76	KB41	BAG		9.8					0.0	14.9	16.3	23.8					
1.52	KB42	BAG		4.3													
2.29	KB43	BAG		4.1													
3.05	KB44	BAG		6.8													
4.57	KB45	BAG		25.5													
6.10	KB46	BAG		28.2													
C		Clifto enginee					PROJEC LOCATI PROJEC	ON	Grassw	rande Su ood, Sas					BOR	E HOL I 105	E NO.

		S	UMN	/IARY	′ OF	SAM	PLIN	G AN	ND LA	BOF	RATO	RY	rest	DAT	Ά		
	SAN	IPLE				CONSI	STENCY			GRAD	ATION			SHEAI	R STR	ENGTH	
DЕРТН	NUMBER	ТҮРЕ	RECOVERY	WATER CONTENT	PLASTIC LIMIT	LIQUID LIMIT	PLASTICITY INDEX	nsc	GRAVEL	SAND	SILT	CLAY	SULPHATE CONTENT	COMPRESSION TEST	LAB VANE	POCKET PEN	DRY DENSITY
meters			mm	%	%	%	%		%	%	%	%	%	kPa	kPa	kPa	kg/m ³
0.76	KB47	BAG		5.2													
1.52	KB48	BAG		4.6													
2.29	KB49	BAG		16.3													
3.05	KB50	BAG		19.1													
4.57	KB51	BAG		26.4													
6.10	KB52	BAG		33.0													
6.10	KB53	SY	260	29.7											120	105	
C				social ience t			PROJE LOCAT PROJE		Grassw		ubdivisio skatchew				BOR	E HOL	E NO.

		S	UMN	/IARY	′ OF	SAM	PLIN	g an	ID LA	BOF	RATO	RY 1	rest	DAT	Ά		
	SAN	IPLE				CONSIS	STENCY			GRAD	ATION			SHEAF	R STRI	ENGTH	
ДЕРТН	NUMBER	ТҮРЕ	RECOVERY	WATER CONTENT	PLASTIC LIMIT	LIMIT	PLASTICITY INDEX	nsc	GRAVEL	SAND	SILT	CLAY	SULPHATE CONTENT	COMPRESSION TEST	LAB VANE	POCKET PEN	DRY DENSITY
meters			mm	%	%	%	%		%	%	%	%	%	kPa	kPa	kPa	kg/m ³
0.76	KB54	BAG		5.9													
1.52	KB55	BAG		5.6													
2.29	KB56	BAG		27.8													
3.05	KB57	BAG		29.0													
4.57	KB58	BAG		31.9													
6.10	KB59	BAG		36.0													
		Clifto enginee				d.	PROJE(LOCATI PROJE(ON	Grassw		ubdivisio skatchew			1	BOR	E HOL 107	E NO.

		S	UMN	/IARY	′ OF	SAM	PLIN	G Al		ABOF	RATO	RY [·]	TEST	DAT	Ά		
	SAN	IPLE				CONSI	STENCY			GRAD	ATION			SHEAR	R STR	ENGTH	
DEPTH	NUMBER	ТҮРЕ	RECOVERY	WATER CONTENT	PLASTIC LIMIT	LIQUID LIMIT	PLASTICITY INDEX	nsc	GRAVEL	SAND	SILT	CLAY	SULPHATE CONTENT	COMPRESSION TEST	LAB VANE	POCKET PEN	DRY DENSITY
meters			mm	%	%	%	%		%	%	%	%	%	kPa	kPa	kPa	kg/m ³
0.76	KB60	BAG		26.1													
1.52	KB61	BAG		36.0			NP	SM	0.0	79.2	20).8					
1.52	KB62	SPT		17.9													
2.29	KB63	BAG		6.7			NP	SM	0.0	79.8	20).2					
2.29	KB64	SPT		9.0													
2.29	KB64B	SPT		35.5													
3.05	KB65	BAG		22.4	19.9	33.7	13.8	CL	0.0	20.6	79).4					
3.05	KB66	SPT		16.4													
4.57	KB67	BAG		22.6													
6.10	KB68	BAG		25.4													
6.71	KB69	BAG		35.1	16.4	51.9	37.3	СН	0.0	3.4	96	6.6					
7.62	KB70	BAG		24.7													
9.14	KB71	BAG		23.0													
10.67	KB72	BAG		25.1													
12.19	KB73	BAG		25.4													
C				social sience t			PROJE LOCAT PROJE	ION		rande Si vood, Sas					BOR	E HOL	E NO.

		S	UMN	/IARY	′ OF	SAM	PLIN	g an	ID LA	BOF	RATO	RY 1	rest	DAT	Ά		
	SAN	IPLE				CONSIS	STENCY			GRAD	ATION			SHEAF	R STR	ENGTH	
DЕРТН	NUMBER	ТҮРЕ	RECOVERY	WATER CONTENT	PLASTIC LIMIT	LIQUID	PLASTICITY INDEX	nsc	GRAVEL	SAND	SILT	CLAY	SULPHATE CONTENT	COMPRESSION TEST	LAB VANE	POCKET PEN	DRY DENSITY
meters			mm	%	%	%	%		%	%	%	%	%	kPa	kPa	kPa	kg/m ³
0.76	KB74	BAG		6.2													
1.52	KB75	BAG		5.4													
2.29	KB76	BAG		10.4													
3.05	KB77	BAG		30.7													
3.05	KB78	SY	230	23.9											N/A	N/A	
4.57	KB79	BAG		36.1													
4.57	KB80	SY	390	22.5											30	35	
6.10	KB81	BAG		25.8													
						<u> </u>											
C		Clifto enginee	n Ass ring sc	social tience t	t es Lt echnolo	d.	PROJE(LOCATI PROJE(ON	Grassw		ubdivisio skatchew				BOR	E HOL I 109	E NO.

		S	UMN	IARY	′ OF	SAM	PLIN	G AN	ND LA	BOF	RATO	RY 1	EST	DAT	Ά		
	SAN	IPLE				CONSI	STENCY			GRAD	ATION			SHEAF	R STR	ENGTH	
DЕРТН	NUMBER	ТҮРЕ	RECOVERY	WATER CONTENT	PLASTIC LIMIT	LIQUID	PLASTICITY INDEX	nsc	GRAVEL	SAND	SILT	СГАҮ	SULPHATE CONTENT	COMPRESSION TEST	LAB VANE	POCKET PEN	DRY DENSITY
meters			mm	%	%	%	%		%	%	%	%	%	kPa	kPa	kPa	kg/m ³
0.76	KB82	BAG		12.8					0.0	25.4	36.2	38.5					
1.52	KB83	BAG		13.4													
2.29	KB84	BAG		22.9													
3.05	KB85	BAG		14.4													
4.57	KB86	BAG		11.7													
6.10	KB87	BAG		26.8													
		Clifto enginee	n Ass ring sc	social ience to	es Lt e echnolo	d. gy	PROJE LOCAT PROJE		Grassw		ubdivisio skatchev				BOR	E HOL I 110	E NO.

		S	UMN	/IARY	′ OF	SAM	PLIN	G AN	ID LA	BOF	RATO	RY 1	EST	DAT	Ά		
	SAN	IPLE				CONSI	STENCY			GRAD	ATION			SHEAR	R STR	ENGTH	
DEPTH	NUMBER	ТҮРЕ	RECOVERY	WATER CONTENT	PLASTIC LIMIT	LIQUID LIMIT	PLASTICITY INDEX	USC	GRAVEL	SAND	SILT	СГАҮ	SULPHATE CONTENT	COMPRESSION TEST	LAB VANE	POCKET PEN	DRY DENSITY
meters			mm	%	%	%	%		%	%	%	%	%	kPa	kPa	kPa	kg/m ³
0.76	MN25	BAG		18.0					0.0	74.6	12.6	12.8					
1.52	MN26	BAG		23.7													
2.29	MN27	BAG		24.4													
3.05	MN28	BAG		26.4													
4.57	MN29	BAG		26.2													
6.10	MN30	BAG		36.1													
7.62	MN31	BAG		35.6													
9.14	MN32	BAG		34.8													
10.67	MN33	BAG		34.9													
12.19	MN34	BAG		33.5													
13.72	MN35	BAG		36.0													
C				social sience t			PROJE LOCAT PROJE		Grassw	rande Si ood, Sas					BOR	E HOL	E NO.

		S	UMN	/IARY	′ OF	SAM	PLIN	g an	ID LA	BOF	RATO	RY 1	rest	DAT	Ά		
	SAN	IPLE				CONSIS	STENCY			GRAD	ATION			SHEAR	R STR	ENGTH	
ДЕРТН	NUMBER	ТҮРЕ	RECOVERY	WATER CONTENT	PLASTIC LIMIT	LIMIT	PLASTICITY INDEX	nsc	GRAVEL	SAND	SILT	CLAY	SULPHATE CONTENT	COMPRESSION TEST	LAB VANE	POCKET PEN	DRY DENSITY
meters			mm	%	%	%	%		%	%	%	%	%	kPa	kPa	kPa	kg/m ³
0.76	MN13	BAG		10.4													
1.52	MN14	BAG		25.1													
2.29	MN15	BAG		34.2													
3.05	MN16	BAG		32.1													
4.57	MN17	BAG		29.5													
6.10	MN18	BAG		31.8													
C		Clifto enginee					PROJEC LOCATI PROJEC	ON	Grassw		ubdivisio skatchew				BOR	E HOL 112	E NO.

		S	UMN	/ IARY	′ OF	SAM	PLIN	G AN	ID LA	BOF	RATO	RY 1	EST	DAT	Ά		
	SAN	IPLE				CONSI	STENCY			GRAD	ATION			SHEAF	R STR	ENGTH	
DЕРТН	NUMBER	ТҮРЕ	RECOVERY	WATER CONTENT	PLASTIC LIMIT	LIQUID	PLASTICITY INDEX	nsc	GRAVEL	SAND	SILT	CLAY	SULPHATE CONTENT	COMPRESSION TEST	LAB VANE	POCKET PEN	DRY DENSITY
meters			mm	%	%	%	%		%	%	%	%	%	kPa	kPa	kPa	kg/m ³
0.76	MN19	BAG		8.4					0.0	32.3	50.4	17.2					
1.52	MN20	BAG		10.6													
2.29	MN21	BAG		15.5													
3.05	MN22	BAG		26.9													
4.57	MN23	BAG		37.3													
6.10	MN24	BAG		36.7													
C		Clifto enginee	n As:	social tience to	es Lt e echnolo	d. gy	PROJE LOCAT PROJE		Grassw		ubdivisio skatchew				BOR	E HOL I 113	E NO.

		S	UMN	/ IARY	′ OF	SAM	PLIN	G AN	ID LA	BOF	RATO	RY 1	rest	DAT	Ά		
	SAN	IPLE				CONSIS	STENCY			GRAD	ATION			SHEAF	R STR	ENGTH	
ДЕРТН	NUMBER	ТҮРЕ	RECOVERY	WATER CONTENT	PLASTIC LIMIT	LIMIT	PLASTICITY INDEX	nsc	GRAVEL	SAND	SILT	CLAY	SULPHATE CONTENT	COMPRESSION TEST	LAB VANE	POCKET PEN	DRY DENSITY
meters			mm	%	%	%	%		%	%	%	%	%	kPa	kPa	kPa	kg/m ³
0.76	MN36	BAG		15.5													
1.52	MN37	BAG		18.1													
2.29	MN38	BAG		29.8													
2.44	MN39A	SY	395	38.1											65	65	
3.05	MN39B	BAG		37.9													
4.57	MN40	BAG		32.4													
6.10	MN41	BAG		28.1													
				social ience to			PROJE LOCAT PROJE		Grassw		ubdivisio skatchew			1	BOR	E HOL	E NO.

		S	UMN	/IARY	′ OF	SAM	PLIN	G AN	ID LA	BOF	RATO	RY 1	rest	DAT	Ά		
	SAN	IPLE				CONSI	STENCY			GRAD	ATION			SHEAF	R STR	ENGTH	
DЕРТН	NUMBER	ТҮРЕ	RECOVERY	WATER CONTENT	PLASTIC LIMIT	LIQUID	PLASTICITY INDEX	nsc	GRAVEL	SAND	SILT	CLAY	SULPHATE CONTENT	COMPRESSION TEST	LAB VANE	POCKET PEN	DRY DENSITY
meters			mm	%	%	%	%		%	%	%	%	%	kPa	kPa	kPa	kg/m ³
0.76	MN58	BAG		6.9									0.01				
1.52	MN59	BAG		22.8									0.14				
2.29	MN60	BAG		33.4													
3.05	MN61	BAG		34.4													
4.57	MN62	BAG		37.7													
6.10	MN63	BAG		32.8													
				social cience t			PROJE LOCAT PROJE		Grassw		ubdivisio skatchew				BOR	E HOL I 115	E NO.

		S	UMN	/ IARY	′ OF	SAM	PLIN	G AN	ID LA	BOF	RATO	RY 1	TEST	DAT	Ά		
	SAN	IPLE				CONSIS	STENCY			GRAD	ATION			SHEAF	R STR	ENGTH	
DЕРТН	NUMBER	ТҮРЕ	RECOVERY	WATER CONTENT	PLASTIC LIMIT	LIQUID LIMIT	PLASTICITY INDEX	nsc	GRAVEL	SAND	SILT	CLAY	SULPHATE CONTENT	COMPRESSION TEST	LAB VANE	POCKET PEN	DRY DENSITY
meters			mm	%	%	%	%		%	%	%	%	%	kPa	kPa	kPa	kg/m ³
0.76	MN1	BAG		19.9									0.74				
1.52	MN2	BAG		23.4									1.11				
2.29	MN3	BAG		29.5													
3.05	MN4	BAG		30.5													
4.57	MN5	BAG		33.4													
6.10	MN6	BAG		30.5													
C		Clifto enginee	n As:	social	echnolo	d. gy	PROJE LOCAT PROJE		Grassw		ubdivisio skatchew				BOR	E HOL I 116	E NO.

		S	UMN	/IARY	′ OF	SAM	PLIN	g an	ID LA	BOF	RATO	RY 1	rest	DAT	Ά		
	SAN	IPLE				CONSIS	STENCY			GRAD	ATION			SHEAF	R STR	ENGTH	
ДЕРТН	NUMBER	ТҮРЕ	RECOVERY	WATER CONTENT	PLASTIC LIMIT	LIQUID	PLASTICITY INDEX	nsc	GRAVEL	SAND	SILT	CLAY	SULPHATE CONTENT	COMPRESSION TEST	LAB VANE	POCKET PEN	DRY DENSITY
meters			mm	%	%	%	%		%	%	%	%	%	kPa	kPa	kPa	kg/m ³
0.76	MN7	BAG		11.2													
1.52	MN8	BAG		5.6													
2.29	MN9	BAG		30.8													
3.05	MN10	BAG		28.6													
4.57	MN11	BAG		34.7													
6.10	MN12	BAG		40.2													
		Clifto enginee				d.	PROJEC LOCATI PROJEC	ON	Grassw		ubdivisio skatchew				BOR	E HOL	E NO.

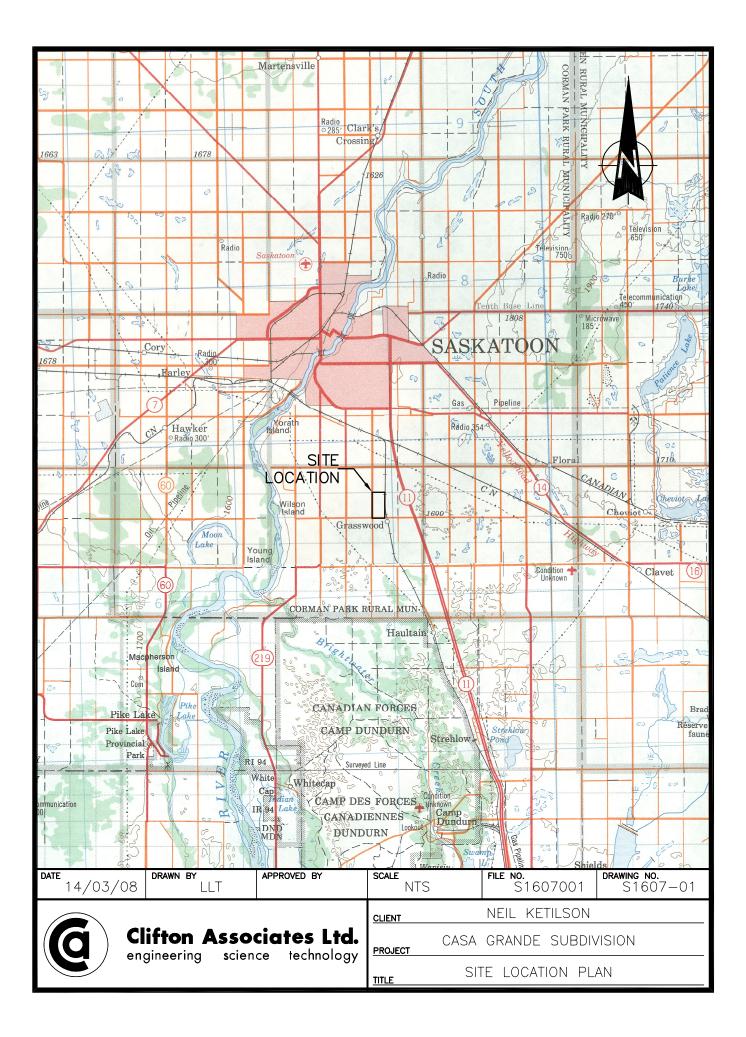
		S	UMN	/IARY	′ OF	SAM	PLIN	G AN	ID LA	BOF	RATO	RY 1	rest	DAT	Ά		
	SAM	IPLE				CONSIS	STENCY			GRAD	ATION			SHEAF	R STR	ENGTH	
ДЕРТН	NUMBER	ТҮРЕ	RECOVERY	WATER CONTENT	PLASTIC LIMIT	LIQUID	PLASTICITY INDEX	nsc	GRAVEL	SAND	SILT	CLAY	SULPHATE CONTENT	COMPRESSION TEST	LAB VANE	POCKET PEN	DRY DENSITY
meters			mm	%	%	%	%		%	%	%	%	%	kPa	kPa	kPa	kg/m ³
0.76	MN52	BAG		6.2													
1.52	MN53	BAG		30.5													
2.29	MN54	BAG		33.3													
3.05	MN55	BAG		31.5													
3.05	MN55A	SY	250	12.2											165	180	
4.57	MN56	BAG		34.0													
6.10	MN56	BAG		22.8													
		<u></u>								<u></u>							
C		Clifto enginee	n Ass ring sc	social ience to	es Lt e echnolo	d. gy	PROJE LOCAT PROJE		Grassw		ubdivisio skatchew				BOR	E HOLI 118	E NO.

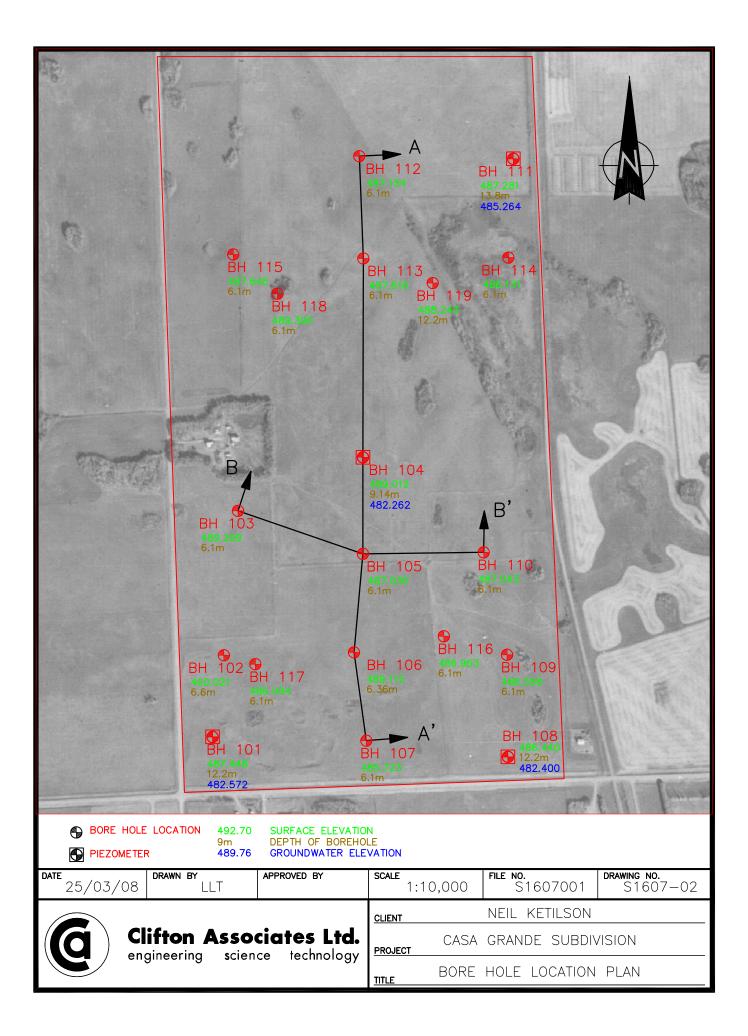
		S	UMN	/ IARY	′ OF	SAM	PLIN	G AN	ND LA	BOF	RATO	RY 1	rest	DAT	Ά		
	SAN	IPLE				CONSIS	STENCY			GRAD	ATION			SHEAF	R STR	ENGTH	
DEPTH	NUMBER	ТҮРЕ	RECOVERY	WATER CONTENT	PLASTIC LIMIT	LIQUID LIMIT	PLASTICITY INDEX	nsc	GRAVEL	SAND	SILT	CLAY	SULPHATE CONTENT	COMPRESSION TEST	LAB VANE	POCKET PEN	DRY DENSITY
meters			mm	%	%	%	%		%	%	%	%	%	kPa	kPa	kPa	kg/m ³
0.76	MN42	BAG		10.2													
1.52	MN43	BAG		9.4													
2.29	MN44	BAG		8.6													
2.74	MN44A	SY	220	2.8											N/A	N/A	
3.05	MN45	BAG		3.7													
4.57	MN46	BAG		23.9													
6.10	MN47	BAG		30.1													
7.62	MN48	BAG		28.0													
9.14	MN49	BAG		26.8													
10.67	MN50	BAG		33.4													
12.19	MN51	BAG		26.7													
C				social ience to			PROJE LOCAT PROJE		Grassw		ubdivisio skatchew				BOR	E HOL	E NO.

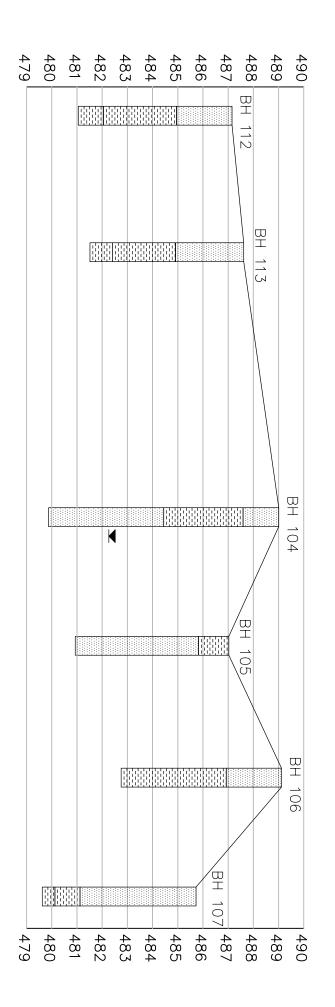




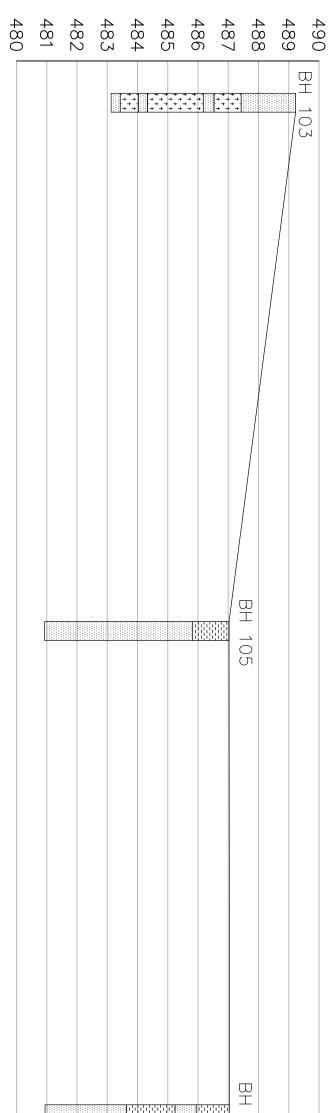
Drawings







SCALE VERTICAL: 1:375,000 HORIZONTAL: 1:7,500



SCALE VERTICAL: 1:50,000 HORIZONTAL: 1:2,500

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6 3 3 1 1 1 1 1 1 1 1 1 1 1 1 1		CLAY SILT SAND
DRAWING REVISIONS Imagineering DESCRIPTION BY Cliffon Associates Lnd. Cliffon Associates Lnd. Cliffon Associates Lnd. States BY BY ML KETILSON BY CROSS SECTION B-B' (WEST-EAST) FLE NO. States States States BY MWE. NO. S160701 S1607-04		





Appendix A

ALS Laboratory Group ANALYTICAL CHEMISTRY & TESTING SERVICES

Environmental Division



		ANALYTICAL REPORT	
ATTN: KIM BONNE	AU		Reported On: 27-FEB-08 03:23 PM
2120 AIRPORT DR.			
SASKATOON SK S	57L 6M6		
Lab Work Order #:	L605119		Date Received: 26-FEB-08
Project P.O. #:			
Job Reference: Legal Site Desc:	S1607		
CofC Numbers:	C061070		
Other Information:			
Comments:			
		Actually	
		NICK PIDSKALNY General Manager, Saskatoon	
	For any questi	ons about this report please contact your	Account Manager:
		RAECHELLE KREESE	

THIS REPORT SHALL NOT BE REPRODUCED EXCEPT IN FULL WITHOUT THE WRITTEN AUTHORITY OF THE LABORATORY. ALL SAMPLES WILL BE DISPOSED OF AFTER 30 DAYS FOLLOWING ANALYSIS. PLEASE CONTACT THE LAB IF YOU REQUIRE ADDITIONAL SAMPLE STORAGE TIME.

ALS Canada Ltd. (formerly ETL Chemspec Analytical Ltd.) Part of the ALS Laboratory Group #819-58th St E., Saskatoon, SK S7K 6X5 Phone: +1 306 668 8370 Fax: +1 306 668 8383 www.alsglobal.com A Campbell Brothers Limited Company

ALS LABORATORY GROUP ANALYTICAL REPORT

Sample Details	s/Parameters	Result	Qualifier*	D.L.	Units	Extracted	Analyzed	Ву	Batch
L605119-1	BH 101								
Sampled By:	NOT PROVIDED on 25-FEB-08 @ 15:00								
Matrix:	WATER								
Poutino M	Vater Analysis								
Alkalinit	-								
	Alkalinity, Total (as CaCO3)	287		5	mg/L	26-FEB-08	26-FEB-08	ANT	R635110
	Bicarbonate (HCO3)	350		5	mg/L	26-FEB-08		ANT	R635110
	Hydroxide (OH)	<5		5	mg/L	26-FEB-08		ANT	R635110
	Carbonate (CO3)	<5		5	mg/L	26-FEB-08	26-FEB-08	ANT	R635110
	Chloride (Cl)	90		1	mg/L	26-FEB-08	26-FEB-08	BFE	R634984
ICP Catio									
	Calcium (Ca)	148		1	mg/L	27-FEB-08	27-FEB-08	DAD	R635292
	Potassium (K)	4.2		0.1	mg/L	27-FEB-08	27-FEB-08	DAD	R635292
	Magnesium (Mg)	73.4		0.1	mg/L	27-FEB-08	27-FEB-08	DAD	R635292
	Sodium (Na)	21		1	mg/L	27-FEB-08	27-FEB-08	DAD	R635292
	Sulfate (SO4)	263		4	mg/L	27-FEB-08	27-FEB-08	DAD	R635292
Ion Bala	nce Calculation								
	Ion Balance	104			%		27-FEB-08		
	TDS (Calculated)	783			mg/L		27-FEB-08		
	Hardness (as CaCO3)	672			mg/L		27-FEB-08		
,	Nitrite and Nitrate+Nitrite-N								_
	Nitrate-N	2.5		0.1	mg/L	26-FEB-08		BFE	R635054
	Nitrite-N	0.08		0.05	mg/L	26-FEB-08		BFE	R635054
	Nitrate+Nitrite-N	2.6		0.1	mg/L	26-FEB-08	26-FEB-08	BFE	R635054
	Conductivity	7.5						0145	D005407
	pH	7.5		0.1	pH	26-FEB-08		CMF	R635197
	Conductivity (EC)	1220		10	uS/cm	26-FEB-08	26-FEB-08	CMF	R635197
L605119-2	BH 108								
Sampled By:	NOT PROVIDED on 25-FEB-08 @ 15:30								
Matrix:	WATER								
Routine V	Vater Analysis								
Alkalinit	y, Total								
	Alkalinity, Total (as CaCO3)	308		5	mg/L	26-FEB-08	26-FEB-08	ANT	R635110
	Bicarbonate (HCO3)	376		5	mg/L	26-FEB-08	26-FEB-08	ANT	R635110
	Hydroxide (OH)	<5		5	mg/L		26-FEB-08	ANT	R635110
	Carbonate (CO3)	<5		5	mg/L	26-FEB-08		ANT	R635110
	Chloride (Cl)	6		1	mg/L	26-FEB-08	26-FEB-08	BFE	R634984
ICP Catio									
	Calcium (Ca)	91		1	mg/L		27-FEB-08	DAD	R635292
	Potassium (K)	3.2		0.1	mg/L	27-FEB-08		DAD	R635292
	Magnesium (Mg)	24.3		0.1	mg/L	27-FEB-08		DAD	R635292
	Sodium (Na)	15		1	mg/L	27-FEB-08		DAD	R635292
	Sulfate (SO4)	42		4	mg/L	27-FEB-08	27-FEB-08	DAD	R635292
	nce Calculation				~		07 555 5		
	Ion Balance	101			%		27-FEB-08		
	TDS (Calculated)	367			mg/L		27-FEB-08		
	Hardness (as CaCO3)	327			mg/L		27-FEB-08		
,	Nitrite and Nitrate+Nitrite-N	<u> </u>						D.C.C.	Dooros
	Nitrate-N	<0.1		0.1	mg/L	26-FEB-08		BFE	R635054
	Nitrite-N	<0.05		0.05	mg/L	26-FEB-08		BFE	R635054
	Nitrate+Nitrite-N	0.1		0.1	mg/L	26-FEB-08	26-FEB-08	BFE	R635054
	Conductivity	7.0		0.1	~니	26 EED 00	26 EED 00	CME	Deserver
	pH Conductivity (EC)	7.3		0.1	pH	26-FEB-08		CMF	R635197
	Conductivity (EC)	640	1	10	uS/cm	20-FEB-08	26-FEB-08	CMF	R635197

ALS LABORATORY GROUP ANALYTICAL REPORT

Sample Details	s/Parameters	Result	Qualifier*	D.L.	Units	Extracted	Analyzed	Ву	Batch
L605119-2	BH 108								
Sampled By:	NOT PROVIDED on 25-FEB-08 @ 15:30								
Matrix: Routine V	WATER Vater Analysis								
	* Refer to Referenced Information for Q	ualifiers (if any) and N	lethodolog	у.					

Reference Information

Methods Listed (if applicable):

ALS Test Code	Matrix	Test Description	Preparation Method Reference(Based On)	Analytical Method Reference(Based On)
ALK-TOT-SK	Water	Alkalinity, Total		APHA 2320 B-Auto-Pot. Titration
Alkalinity is determined the hydroxide (if present) also		of an aliquot with standar	dized acid solution to a pH of 4.5. Total alkalini	ty, bicarbonate, carbonate(if present) and
Reference Greenberg, Arnold E., C Method 2320B.	leseri, Leno	re S., Eaton, Andrew D., S	Standard Methods For The Examination of Wate	er and Wastewater, 18th Edition, 1992,
CL-SK	Water	Chloride (Cl)		APHA 4500 CL-E
			h by complexation with mercury (II) thiocynate. Ins a highly colored ferric thiocyanate complex.	In the colorimetric method, chloride (CI-)
Reference Greenberg, Arnold E., C Method 4500Cl-E.	leseri, Leno	re S., Eaton, Andrew D., S	Standard Methods For The Examination of Wate	er and Wastewater, 18th Edition, 1992,
ETL-ROUTINE-ICP-SK	Water	ICP Cations		APHA 3120 B-ICP-OES
These ions are determin	ed directly y	ICP-OES.		
Reference Greenberg, Arnold E., C Method 3120B.	leseri, Leno	re S., Eaton, Andrew D., S	Standard Methods For The Examination of Wate	er and Wastewater, 18th Edition, 1992,
IONBALANCE-SK	Water	Ion Balance Calculation		APHA 1030E
N2/N3-SK	Water	Nitrate, Nitrite and		APHA 4500 NO3F
nitrite) is then determine water-soluble dye has a	d by diazoti magenta co	zing with sulfanilamide foll	mple through a copperized cadmium column. owed by coupling with N-(1-naphthyl)ethylened 520nm. Original nitrite can also be determined O2-N are reported.	iamine dihydrochloride. The resulting
Reference Greenberg, Arnold E., C Method 4500NO3-F.	leseri, Leno	re S., Eaton, Andrew D., S	Standard Methods For The Examination of Wate	er and Wastewater, 18th Edition, 1992,
PH/EC-SK	Water	pH and Conductivity		APHA 4500-H, 2510
			** Laboratory Methods employed follow generally based on nationally or internat	
Chain of Custody numb	ers:			
C061070				
The last two letters of th	e above tes	t code(s) indicate the labo	ratory that performed analytical analysis for the	at test. Refer to the list below:
Laboratory Definition Co	de Lab	pratory Location	Laboratory Definition Code	Laboratory Location
SK	SAS	LABORATORY GROUP - KATOON, SASKATCHEW ADA		

Reference Information

GLOSSARY OF REPORT TERMS

Surr - A surrogate is an organic compound that is similar to the target analyte(s) in chemical composition and behavior but not normally detected in environmental samples. Prior to sample processing, samples are fortified with one or more surrogate compounds. The reported surrogate recovery value provides a measure of method efficiency. The Laboratory control limits are determined under column heading D.L.

mg/kg (units) - unit of concentration based on mass, parts per million.

mg/L (units) - unit of concentration based on volume, parts per million.

< - Less than.

D.L. - The reporting limit.

N/A - Result not available. Refer to qualifier code and definition for explanation.

Test results reported relate only to the samples as received by the laboratory. UNLESS OTHERWISE STATED, ALL SAMPLES WERE RECEIVED IN ACCEPTABLE CONDITION. UNLESS OTHERWISE STATED, SAMPLES ARE NOT CORRECTED FOR CLIENT FIELD BLANKS. Although test results are generated under strict QA/QC protocols, any unsigned test reports, faxes, or emails are considered preliminary.

ALS Laboratory Group has an extensive QA/QC program where all analytical data reported is analyzed using approved referenced procedures followed by checks and reviews by senior managers and quality assurance personnel. However, since the results are obtained from chemical measurements and thus cannot be guaranteed, ALS Laboratory Group assumes no liability for the use or interpretation of the results.

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