

Subdivision Development Investigation
Casa Grande
(NW & SW 26-35-5 W3M)
Grasswood, Saskatchewan

File S1607

29 August 2008

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Bore hole Nos. 101 to 119 inclusive

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1.0 Introduction

1.1 Background

This report presents results of the investigation conducted for the proposed Casa Grande subdivision located south of Saskatoon on Preston Avenue. A site location plan is presented in Drawing No. S1607-01. The legal description of the area is NW and SW26-35-5-W3M. The proposed subdivision would consist of 70 – 80 lots ranging in size from 1 – 5 acres on both quarters being considered for development. The site is primarily used as pastureland. There are currently two residences located on each quarter section.

No previous geotechnical and hydrogeological investigations have been performed at the proposed Casa Grande subdivision development.

1.2 Objectives

The objectives of the subdivision investigation were to evaluate slope stability, determine wastewater disposal characteristics, and to provide preliminary foundation and construction recommendations based on a geotechnical investigation.

The investigation is to provide a preliminary assessment of site conditions which will be a first step in developing data to support subsequent applications to regulators. The level of detail is intended to provide basic site characterization. Further detail may be required for regulators such as Saskatchewan Health or Saskatchewan Environment.

1.3 Scope of Work

The scope of the investigation included:

- Compilation of local and regional geological information for the area;
- Assessment of the stratigraphy and hydrology at the site;
- Visual investigation of the site and aerial photograph analysis for evidence of slope instability;
- Preliminary geotechnical and hydrogeological investigations to assist with permit applications for installation and construction of wastewater disposal systems;

- Foundation recommendations and restrictions arising from the geotechnical investigation; and,
- Reporting, including stratigraphic cross-sections identifying the geology and definition of the piezometric surface of the site.

1.4 Existing Information

Various sources of information are available which were used to develop a general assessment of the geological and hydrogeological features of the subject site and its surrounding area. The following information was used for an assessment of the area around this site:

- Christiansen, E.A., 1970. Physical Environment of Saskatoon, Canada
- Saskatchewan Geomatics aerial photographs, 1990
- SaskWater Well Data provided by SaskWater

2.0 Physical Environment

2.1 Regional Geology

The bedrock surface in the region consists of the Cretaceous Bearpaw Formation which is overlain by a succession of Quaternary deposits of till and stratified drift from the Saskatoon Group. The Bearpaw Formation is the youngest bedrock in the area, and has a varying thickness near the study area of 45 to 62 m thick. It is a non-calcareous, silt and clay. The uppermost glacial deposit consist of the Saskatoon Group that includes the Floral and Battleford Formations and the surficial stratified drift deposits. In the area the Floral Formation is absent along with the Sutherland Group. The Saskatoon Group Formation is approximately 100 m to 110 m thick and consists of clay till. The surficial stratified drift deposits consist of stratified silt, clay, sands and gravels.

2.2 Regional Hydrogeology

The mapped aquifers are mainly surficial stratified deposits. The Moose Woods Flats Aquifer is the most extensive aquifer in the region. The aquifer at the site is approximately 37 m bgs. A search of the SaskWater Corporation Database (current to May 2007) indicated 133 water withdrawal well and five water test hole records within a one mile radius of the site. These

records provided a good sample of water use for the area. The majority of the boreholes were complete below 5.5 m bgs in surficial deposits and sand layers and lenses present in the Saskatoon Group. The borehole lithology logs showed that the stratigraphy in the area consisted primarily of sand, silt and clay layers.

3.0 Field and Laboratory Investigation

Subsurface geology was investigated by a total of 19 test borings on the site, designated as Bore Holes 101 to 119. The locations of bore holes are shown on Drawing No. S1607-02. Bore holes were drilled to a depth of 6 m to 20 m. Drilling was conducted on 17 January and 8 February 2008 using a truck-mounted Brat drill rig and 125 mm solid stem continuous flight auger. Bore holes were logged and sampled at a 1.5 m interval.

Piezometers were installed in Bore Holes 101, 104, 108, and 111. Water levels in piezometers were measured on 25 February 2008.

Moisture contents were determined for all samples, and Unified Soil Classifications and particle size analyses were performed on select representative samples.

Observations made during the field investigations, visual descriptions and the results of laboratory tests are recorded in the Bore hole Logs and Summary of Sampling and Laboratory Test Data, and are appended to this report. An explanation of the symbols and terms used in the bore hole logs is included in the Symbols and Terms section of this report.

3.1 Stratigraphy

The site is located on a glacial lake basin, with subsurface soil consisting primarily of sand and silt with some clay. Some dune sand is present along the southern edge of SW26.

Stratigraphy consisted of stratified sand, silt and clay. Table 3.1 provides a summary of index properties of soil encountered, including moisture content and results of Atterberg limits and Unified Soil Classification testing. Silty sand covered the majority of the site to varying depth. Sand was generally moist and compact, with standard penetration testing 'N' values of 9 to 16 blows for 300 mm penetration.

Clay strata varied in thickness, and generally possessed medium to high plasticity. It was moist and stiff to very stiff in consistency, with an undrained shear strength of about 160 kPa.

Silt and clayey silt strata with some sand were encountered in some areas.

Table 3.1
Index Properties of Representative Samples

Sample Tested	Natural Water Content (%)	Plastic Limit	Liquid Limit	Plasticity Index	Unified Soil Classification*
BH101 @ 0.8 to 1.0 m	5.3	NP	NP	NP	SM
BH101 @ 1.5 to 1.8 m	25.8	23.9	68.2	44.3	CH
BH101 @ 2.3 to 2.4 m	9.8	NP	NP	NP	SM
BH101 @ 4.6 to 4.8 m	17.8	NP	NP	NP	SM
BH101 @ 7.5 to 7.6 m	34.0	23.6	57.9	34.3	CH
BH101 @ 10.6 to 10.7 m	38.9	28.7	77.9	49.2	CH
BH101 @ 12.1 to 12.2 m	34.6	24.0	74.6	50.6	CH
BH104 @ 0.7 to 0.8 m	9.2	NP	NP	NP	SM
BH104 @ 1.4 to 1.5 m	11.8	19.5	43.3	23.8	CL
BH104 @ 2.2 to 2.3 m	16.5	18.9	49.1	30.2	CL
BH104 @ 3.1 to 3.4 m	23.4	26.4	63.7	37.3	CH
BH104 @ 4.6 to 4.8 m	9.1	NP	NP	NP	SM
BH104 @ 7.5 to 7.6 m	28.7	NP	NP	NP	SM
BH108 @ 1.4 to 1.5 m	36.0	NP	NP	NP	SM
BH108 @ 2.2 to 2.3 m	6.7	NP	NP	NP	SM
BH108 @ 3.0 to 3.1 m	22.4	19.9	33.7	13.8	CL
BH108 @ 6.6 to 6.7 m	35.1	16.4	51.9	37.3	CH

*CL – low plasticity clay, CH- high plasticity clay, SM – silty sand, NP- non-plastic

3.2 Groundwater

Groundwater levels were measured on 25 February 2008. The water elevation in each piezometer is presented in Table 3.2 and on Drawing No. S1604-02. Groundwater elevations were utilized to determine the horizontal hydraulic gradient. The groundwater flow direction at the site was determined to be towards the southeast.

Table 3.2
Water Elevations

Piezometer	Water Elevation (m)	Ground Elevation (m)	Casing Elevation (m)	Depth to Water (m)
BH101	482.572	487.448	488.412	4.88
BH104	482.262	489.012	489.896	6.75
BH108	482.400	486.440	487.390	4.04
BH111	485.264	487.281	488.094	2.02

4.0 Slope Stability

Landforms in the area were defined on the basis of aerial photography. An aerial photograph showing the site is presented in Drawing No. S1607-2. This area was part of Glacial Lake Saskatoon during the last deglaciation. While under water, sand, silt and clay was deposited. Sand dunes are present along the southern edge of the area being developed. The site has approximately 5 m of relief. Large scale landsliding is not an issue in this area.

5.0 Wastewater Disposal

5.1 Scope

The site was assessed in terms of the geotechnical and hydrogeological site characteristics required to install wastewater disposal systems as per the regulations and guidelines set out in the *Saskatchewan Onsite Wastewater Disposal Guide* (First Edition, 2007, Saskatchewan Health), and the *Onsite Wastewater Management: Review Process for Developments and Subdivisions* (Saskatoon Health Region, Public Health Services). These documents will be referenced as SOWDG and OWM, respectively, for the purposes of this report.

5.2 Regulatory Requirements

The proposed development falls within the High Sensitivity Area section as per the OWM. As such, only holding tanks, pressure chamber systems, package sewage treatment plants and Type II Mounds will be permitted at the Casa Grande site due to the proposed size and number of lots on each quarter section. It also states that any existing wastewater disposal

systems in use or intended for use would need to be upgraded to comply with current requirements for high sensitivity developments.

The SOWDG states that there should be a minimum isolation distance of 1.5 m between a wastewater disposal system and the water table. All setback requirements listed in the SOWDG must also be met. Replacement disposal areas, if ever needed, should be located adjacent to the existing disposal area.

The SOWDG states that a Type II mound may be constructed on a natural slope provided that:

- The slope is less than 3% and the percolation rate is not slower than 60 minutes per 25 mm to a depth of at least 600 mm below the sand layer;
- The slope is less than 6% and the percolation rate is not slower than 30 minutes per 25 mm to a depth of at least 600 mm below the sand layer;
- The slope does not exceed 12% regardless of percolation rate.

A chamber system may also be constructed on a slope provided that distribution devices or step-downs are used.

Package sewage treatment plants such as three-cell septic tanks provide a greater level of treatment therefore may allow for a reduction in the treatment area required; however, they are not mandatory in a High Sensitivity Area.

5.3 Soil Loading Rates

As per the SOWDG, wastewater disposal systems are sized based on the soil loading rate. Soil loading rates are determined either via percolation testing or soil texture classification. Soil texture classification was used to determine appropriate soil loading rates for the various surficial soil types encountered at the proposed Casa Grande subdivision.

5.3.1 Soil Texture Classification

The results of the particle size analysis and hydrometer testing performed on select samples that are representative of the material in the upper strata are presented in Table 5.1. Based on the percentage of silt and clay versus sand, the soil was classified as per the Soil Texture Classification Triangle in Appendix 15 of the SOWDG.

Table 5.1
Summary of Particle Size Analysis
and Soil Texture Classification of Upper Soil Units

Soil Type	Sample Number	Gravel (%)	Sand (%)	Silt (%)	Clay (%)	Soil Texture as per SOWDG
Sand	KB13	0.0	76.7	13.2	10.1	Sandy Loam
Silty Clay	KB41	0.0	14.9	16.3	23.8	Silt Loam
Clay	KB82	0.0	25.4	36.2	38.5	Clay Loam
Silt	MN19	0.0	32.3	50.4	17.2	Loam
Sand	MN25	0.0	74.6	12.6	12.8	Sandy Loam

The corresponding loading rates as per Appendix 15 of the SOWDG can be applied to size the wastewater disposal systems:

- Clay Loam (Clay) 10.78 L/m²
- Silt Loam (Silty Clay) 13.72 L/m²
- Loam (Silt) 17.15 L/m²
- Sandy Loam (Sand) 22.05 L/m²

It is important to note that the soil texture classifications provided in this report are based upon a single hydrometer test for each surficial soil unit encountered. Also, the classifications do not account for secondary structure within the soil unit such as fracturing, which can greatly increase the permeability of a soil.

5.4 Groundwater

Water level measurements taken at the site (Table 3.2) indicate that the water table at the site ranges from 6.75 m to 2.02 m below ground surface, which ranges in elevation from 485.26 m to 482.26 m.

Assuming an average water table location of about 2 to 5 m below ground surface, the minimum separation distance of 1.5 m will be met.

A water sample was taken from the standpipe piezometers in BH101 and 108 and analyzed for routine water chemistry to obtain background groundwater data. The results are presented in Appendix A.

5.5 Conclusions

The following recommended wastewater disposal systems for the proposed development are described in relation to the soil type of the disposal area:

- Type II mound systems with two-cell septic tanks would provide adequate wastewater disposal for areas where the surficial material is sand, silt, or clay.
- Type II mound systems, with three-cell septic tanks would provide adequate wastewater disposal for areas where the surficial material is sand, silt, or clay.

The soil loading rate used to size the selected system should be determined based on the lowest soil loading rate of any of the materials encountered in the upper 900 mm of soil (excluding topsoil).

6.0 Foundation Design and Construction Recommendations

It is our understanding that the subdivision will primarily consist of single family dwellings. Structures will likely be one or two storeys over a full basement with a grade supported concrete floor. Geotechnical issues associated with this type of structure are foundations to support the proposed structures and construction of a grade supported floor on a variable subgrade that could be silt, sand or high plasticity clay. Groundwater levels measured were 2.0 m to 6.8 m below existing ground surface (Table 3.2), and vary in elevation from 485.26 m to 482.26 m. Groundwater levels can be expected to vary with time, and may increase as a result of development.

Although fill material was not noted during this field investigation, its presence and condition should be noted during construction since it is not desirable to place foundations or floors on fill material of unknown composition and consistency. Foundations or floors should not be constructed on organic topsoil or organic soil.

6.1 Waterproofing and Subdrainage

The quantity of seepage and groundwater levels will vary seasonally, with precipitation or snowmelt, and with development due to irrigation and other factors. Rates of flow can be relatively high through sand and sandy strata. Groundwater levels are variable across the site, and were at least 2.0 m below ground surface.

Typically, basement floors will be about 1.5 m to 2.0 m below finished grade. On this basis, it is not likely that hydrostatic pressures will develop on basement walls and floors. However, basement walls can be waterproofed to accommodate any future increases in groundwater levels that could lead to seepage into basements. At a minimum, walls must be damp-proofed; floors should be damp-proofed.

A perimeter subdrainage system should be installed at the base of the footing. This requirement can be reviewed depending on specific conditions for any home. The excavation should be backfilled with a free draining granular soil to within about 0.6 m of surface to ensure that water can freely drain to a perimeter weeping tile system. Free draining means that there is less than 3 percent silt and clay particles. Clay or clayey soil can be placed on the surface to reduce the amount of infiltration.

6.2 Foundations

The proposed structures may be supported on shallow spread footings or augered cast-in-place concrete piles. Lightly loaded structures supported on shallow spread footings on medium to high plasticity clay will experience some vertical movement associated with changes in soil moisture. Total vertical movement is estimated to be as much as 150 mm for foundations on medium to high plasticity clay, with differential movement less than half of the total vertical movement. The anticipated vertical movement for foundations on sand will typically be less than about 25 mm.

Assuming that the basement or crawlspace will be insulated, the footing must be constructed below the anticipated depth of frost, estimated to be about 1.8 m in this area. This depth can be reduced on the basis of local experience and on the performance of similar foundations in the area. The foundation should not be allowed to freeze, particularly during construction, as frost heave may occur.

The allowable bearing capacity for a shallow spread footing will depend on the type of soil at the footing elevation. The information provided in this report is for preliminary purposes, only, and should not be relied upon for detailed design due to the variation in conditions across the site. Site specific investigations are recommended for buildings on this development. Foundation conditions, soil type, and allowable bearing capacity should be confirmed for specific sites. For preliminary design, the allowable bearing capacity for a spread footing constructed on compact sand will be 100 kPa (2,100 lb/ft²). For shallow spread footings constructed on stiff clay, the same value can be used. Sand encountered at the foundation elevation should be well compacted to minimize the potential for settlement. If

sand or sandy soil is wet and excessive pumping is encountered during compaction, the sand may be subcut 300 mm or more and replaced with a well graded, pit run material. A geotextile may be used as a separator at the base of the fill to reduce pumping of fines up into the fill, while allowing water to escape.

Structures can be supported on augered cast-in-place concrete piles designed on the basis of skin friction. However, standup conditions in sand will not be good and sleeving will likely be necessary to keep excavations open for concreting. Settlement of piles is expected to be less than 5 mm for a properly designed and constructed pile foundation. The skin friction contribution of the upper 2.0 m of pile below finished grade should be ignored in the determination of pile capacity for perimeter piles supporting a grade beam. This can be reduced to 1.0 m for interior piles.

An allowable skin friction value of 25 kPa may be used for design of piles in clay. A value of 15 kPa may be used where sand is encountered. The minimum length of pile should be 5 m. Grade beams should be constructed with a minimum 100 mm void space so that any heaving of the subgrade soil does not exert an upward force on piles, which can result in separation of the grade beam from the pile and distortion of the structure.

For augered piles, concrete should be placed within 2 hrs of excavation to minimize softening of clay or silt which can reduce pile capacity, or excessive sloughing and squeezing of soil, which can result in necking of the pile. The aspect ratio of a pile, defined as the ratio between length and diameter, should not exceed about 30. This should ensure that good contact is maintained between the concrete and soil and that no voids are created.

The use of water to facilitate excavation of piles should be avoided, since this will result in softening of the soil in contact with the concrete, reducing pile capacity. Inspection during construction is suggested to ensure compliance with specifications.

Landscaping around the structures should consider potential effects on foundation performance. Plantings of trees and large shrubs immediately adjacent the foundation should be avoided. Grading around the building should ensure positive drainage. Care should be taken to ensure that downspouts divert water away from the foundation.

6.3 Floors

Floors placed on a medium to high plasticity clay subgrade will experience some vertical movement associated with heave or shrinkage due to changes in soil moisture. The presence

of fill material of unknown or variable type and consistency may result in differential settlement of a grade supported floor. It is estimated that as much as 150 mm of heave may occur for floors constructed on medium to high plasticity clay. Little vertical movement is expected for a well constructed floor constructed on a sand subgrade.

The specification for compaction of clay subgrade soil should specifically indicate that the water content should be at optimum to optimum + 2 percent, since clay compacted wet of optimum will have a lower potential for heave. This will not, however, eliminate the potential for heave.

6.4 Potential for Sulphate Attack

The water soluble sulphate content of soil was measured to be 0.01 percent by dry weight of soil in sand and 0.14 percent in clay. On this basis, the potential for sulphate attack will be moderate to severe for concrete in contact with clay. Sulphate resistant (Type 50 or HS) cement must be specified for all concrete in contact with clay soil. Recommendations regarding sulphate resistant cement may be found in CSA A23.1.

7.0 Closure

This report was prepared by Clifton Associates Ltd. for the use of Mr. Neil Ketilson and his agents for specific application to the proposed Casa Grande subdivision south of Saskatoon. The material in it reflects Clifton Associates Ltd. best judgment available to it at the time of preparation. Any use which a third party makes of this report, or any reliance on or decisions to be made based on it, are the responsibility of such third parties. Clifton Associates Ltd. accepts no responsibility for damages, if any, suffered by any third party as a result of decisions made or actions based on this report.

This report has been prepared in accordance with generally accepted engineering practice common to the local area. No other warranty, expressed or implied is made

Our conclusions and recommendations are preliminary and based upon the information obtained from the referenced subsurface exploration. The boreholes and associated laboratory testing indicate subsurface and groundwater conditions only at the specific locations and times investigated, only to the depth penetrated and only for the soil properties tested. The subsurface conditions may vary between the boreholes and with time. The subsurface interpretation provided is a professional opinion of conditions and not a certification of the site conditions. The nature and extent of subsurface variation may not become evident until

construction or further investigation. If variations or other latent conditions do become evident, Clifton Associates Ltd. should be notified immediately so that we may re-evaluate our conclusions and recommendations. Although subsurface conditions have been explored, we have not conducted analytical laboratory testing on samples obtained nor evaluated the site with respect to the potential presence of contaminated soil or groundwater conditions.

The enclosed report contains the results of our investigations as well as certain recommendations arising out of such investigations. Our recommendations do not constitute a design, in whole or in part, of any of the elements of the proposed work. Incorporation of any or all of our recommendations into the design of any such element does not constitute us as designers or co-designers of such elements, nor does it mean that such design is appropriate in geotechnical terms. The designers of such elements must consider the appropriateness of our recommendations in the light of all design criteria known to them, many of which may not be known to us. Our mandate has been to investigate and recommend which we have completed by means of this report. We have had no mandate to design, or review the design of, any elements of the proposed work and accept no responsibility for such design or design review.

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Symbols and Terms

Soil Descriptive Terms

A soil description for geotechnical applications includes a description of the following properties:

- texture
- color, oxidation
- consistency and condition
- primary and secondary structure

Texture

The soil texture refers to the size, size distribution and shape of the individual soil particles which comprise the soil. The Unified Soil Classification System (ASTM D2487-00) is a quantitative method of describing the soil texture. The basis of this system is presented overleaf. The following terms are commonly used to describe the soil texture.

Particle Size (ASTM D2487-00)		Relative Proportions (CFEM, 3rd Ed., 1992)	
Boulder	300 mm plus	Trace	1 - 10 %
Cobble	75 - 300 mm	Some	10 - 20 %
Gravel	4.75 - 75 mm	Gravelly, sandy, silty, clayey, etc.	20 - 35 %
Coarse	19 - 75 mm		
Fine	4.75 - 19 mm		
Sand	0.075 - 4.75 mm	And	>35 %
Coarse	2 - 4.75 mm		
Medium	0.425 - 2 mm		
Fine	0.075 - 0.425 mm	Gravel, Sand, Silt, Clay	>35 % and main fraction
Silt and Clay	Smaller than 0.075 mm		

Gradation		Particle Shape	
Well Graded	Having a wide range of grain sizes and substantial amount of all intermediate sizes.	Angular	Sharp edges and relatively plane sides with unpolished surfaces.
Uniform or Poorly Graded	Possessing particles of predominantly one size.	Subangular	Similar to 'angular' but have rounded edges.
Gap Graded	Possessing particles of two distinct sizes.	Subrounded	Well-rounded corners and edges, nearly plane sides.
		Rounded	No edges and smoothly curved sides.
		Also may be flat, elongated or both.	

The term "TILL" may be used as a textural term to describe a soil which has been deposited by glaciers and contains an unsorted, wide range of particle sizes.

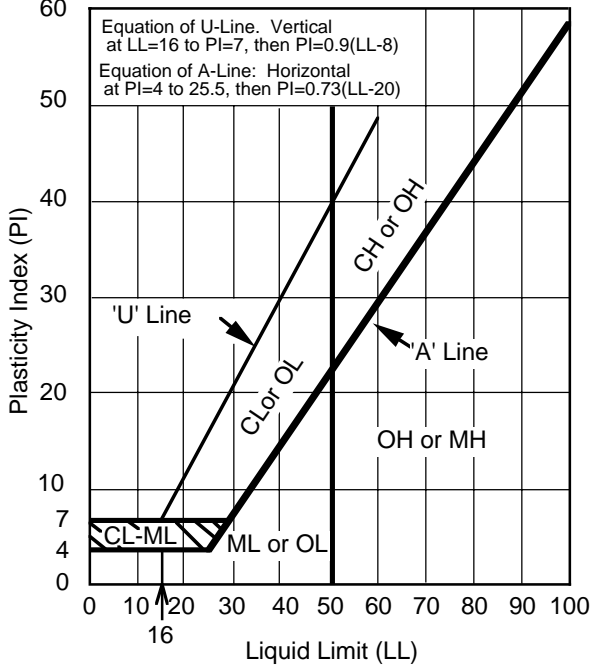
Color And Oxidation

The soil color at its natural moisture content is described by common colors and, quantitatively, in terms of the Munsell color notation; (eg. 5Y 3/1). The notation combines three variables, hue, value and chroma to describe the soil color. The hue indicates its relation to red, yellow, green, blue and purple. The value indicates its lightness. The chroma indicates its strength of departure from a neutral of the same lightness.

Departure of the soil color from a neutral color indicates the soil has been oxidized. Oxidation of a soil occurs in a oxygen rich environment where most commonly metallic iron, oxidizes and turns a neutral colored soil 'rusty' or reddish brown. Oxidized manganese gives a purplish tinge to the soil. Oxidation may occur throughout the entire soil mass or on fracture/joint/fissure surfaces.

Classification of Soils for Engineering Purposes

ASTM Designation D 2487-00 (Unified Soil Classification System)

Major divisions		Group Symbols	Typical names		Classification criteria						
Coarse-grained soils More than 50% retained on No. 200 sieve* (>0.075 mm)	Gravels More than 50% of coarse fraction retained on No. 4 sieve(≥4.75 mm)	Clean gravels <5% fines	GW	Well-graded gravel	If ≥ 15% sand add "with sand" to group name	$C_u = \frac{D_{60}}{D_{10}} \geq 4;$ $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3					
			GP	Poorly graded gravel		Not meeting either C_u or C_c criteria for GW					
		Gravels with fines >12% fines	GM	Silty gravel		Atterberg limits below "A" line or PI less than 4					
			GC	Clayey gravel		Atterberg limits on or above "A" line and PI > 7					
	Sands 50% or more of coarse fraction passes No. 4 sieve(<4.75 mm)	Clean sands <5% fines	SW	Well-graded sand	If ≥ 15% gravel add "with gravel to group name	$C_u = \frac{D_{60}}{D_{10}} \geq 6;$ $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3					
			SP	Poorly graded sand		Not meeting either C_u or C_c criteria for SW					
		Sands with fines >12% fines	SM	Silty sand		Atterberg limits below "A" line or PI less than 4					
			SC	Clayey sand		Atterberg limits on or above "A" line and PI > 7					
Classification on basis of percentage of fines Less than 5% pass No. 200 sieve.....GW, GP, SW, SP More than 12% pass No. 200 sieve.....GM, GC, SM, SC 5 to 12% pass No. 200 sieve.....Borderline classifications requiring use of dual symbols											
Fine-grained soils 50% or more passes No. 200 sieve* (≤0.075 mm)	Silts and Clays Liquid limit <50%	Inorganic	ML	Silt	If 15 to 29% coarse-grained, add "with sand" or "with gravel" as appropriate If > 30% coarse-grained, add "sandy" or "gravelly" as appropriate Class as organic when oven dried liquid limit is < 75% of undried liquid limit	<div>Plasticity Chart</div>  <p>Equation of U-Line. Vertical at LL=16 to PI=7, then PI=0.9(LL-8) Equation of A-Line: Horizontal at PI=4 to 25.5, then PI=0.73(LL-20)</p>					
			CL	Lean Clay -low plasticity							
		Organic	OL	Organic clay or silt (Clay plots above 'A' Line)							
			Inorganic	MH					Elastic silt		
	CH	Fat Clay -high plasticity									
	Organic	OH		Organic clay or silt (Clay plots above 'A' Line)							
		Highly organic soils	PT	Peat, muck and other highly organic soils							

*Based on the material passing the 3 in.(75 mm) sieve, if field samples contain cobbles or boulders, add "with cobbles or boulders" to group name

Consistency And Condition

The consistency of a cohesive soil is a qualitative description of its resistance to deformation and can be correlated with the undrained shear strength of the soil. The condition of a coarse grained soil qualitatively describes the soil compactness and can be correlated with the standard penetration resistance (ASTM D1586-99).

Consistency Of Cohesive Soil (CFEM, 3rd Edit., 1992)

Consistency	Undrained Shear Strength (kPa) (CFEM, 3rd Edit., 1992)	Field Identification (ASTM D 2488-00)
Very Soft	<12	Thumb will penetrate soil more than 25 mm.
Soft	12-25	Thumb will penetrate soil about 25 mm.
Firm	25-50	Thumb will indent soil about 6 mm.
Stiff	50-100	Thumb will indent, but penetrate only with great effort (CFEM).
Very Stiff	100-200	Readily indented by thumbnail (CFEM).
Hard	>200	Thumb will not indent soil but readily indented with thumbnail.
Very Hard	N/A	Thumbnail will not indent soil.

Condition Of Coarse Grained Soil (CFEM, 3rd Edit., 1992)

Compactness Condition	SPT N - Index (Blows/300mm)
Very Loose	0 - 4
Loose	4 - 10
Compact	10 - 30
Dense	30 - 50
Very Dense	over 50

Moisture Conditions (ASTM D2488-00)

Description	Criteria
Dry	Absence of moisture, dusty, dry to touch
Moist	Damp but no visible water
Wet	Visible, free water, usually soil is below water table

Structure

The soil structure is the manner in which the individual soil particles are assembled to form the soil mass. The primary soil structure is the arrangement of soil particles as originally deposited. The secondary soil structure refers to any rearrangement of the soil such as deformation and cracking which has taken place since deposition.

Primary Soil Structure (Depositional)

A. Geometry

Stratum	- A single sedimentary 'layer', greater than 10 mm in thickness, visibly separable from other strata by a discrete change in lithology and/or sharp physical break.
Homogeneous	- Same color and appearance throughout.
Stratified	- Consisting of a sequence of layers which are generally of contrasting texture or color.
Laminated	- Stratified with layer thicknesses between 2 mm and 10 mm.
Thinly laminated	- Stratified with layer thickness less than 2 mm.
Bedded	- Stratified with layer thicknesses greater than 10 mm.
Very Thinly Bedded (Flaggy)	- Stratified with layer thicknesses between 10 and 50 mm.
Thinly Bedded (Slabby)	- Stratified with layer thicknesses between 50 and 600 mm.
Thickly Bedded (Blocky)	- Stratified with layer thicknesses between 600 and 1200 mm.
Thick-Bedded (Massive)	- Stratified with layer thicknesses greater than 1200 mm.
Lensed	- Inclusions of small pockets of different soils, such as small lenses of sand material throughout a mass of clay.

B. Bedding Structures

Cross-bedding	- Internal 'bedding' inclined to the general bedding plane.
Ripple-bedding	- Internal 'wavy bedding'.
Graded-bedding	- Internal gradation of grain size from coarse at base to finer at top of bed.
Horizontal bedded	- Internal bedding is parallel and flat lying

Secondary Soil Structure (Post-Depositional)

A. Accretionary Structures

Includes nodules, concretions, crystal aggregates, veinlets, color banding and


















Cementation	- Chemically precipitated material, commonly calcite (CaCO_3), binds the grains of soil, usually sandstone. Described as weak, moderate, strong (ASTM D2488-00).
Salt Crystals	- Groundwater flowing through the soil/rock often precipitates visible amounts of salts. Calcite (CaCO_3), glauber salts ($\text{Na}_2\text{Ca}(\text{SO}_4)_2$), and gypsum ($\text{CaSO}_4 \cdot 2\text{H}_2\text{O}$) are common.

B. Fracture Structures










Fracture	- A break or discontinuity in the soil or rock mass caused by stress exceeding the materials strength.
Joint	- A fracture along which no displacement has occurred.
Fissure	- A gapped fracture, which may open and close seasonally. Usually an extensive network of closely spaced fractures, giving the soil a 'nuggetty' structure.
Slickensides	- Fractures in a clay that are slick and glossy in appearance, caused by shear movements.
Brecciated	- Contains randomly oriented angular fragments in a finer mass, usually associated with shear displacements in soils.
Fault	- A fracture or fracture zone along which there has been displacement.
Blocky	- A cohesive soil that can be broken down into small angular lumps which resist further breakdown.

Symbols Used on Bore Hole Logs






Lithology Type

	CLAY		TILL-oxidized		COAL		CLAY SHALE
	SILT		TILL-unoxidized		FILL (Undifferentiated)		SANDSTONE
	SAND		PEAT		CONCRETE		MUDSTONE
	GRAVEL		TOPSOIL or ORGANIC SOIL		ASPHALT		BEDROCK (Undifferentiated)
	COBBLES						



Borehole Completion and Backfill Materials

	Bentonite		Cuttings		Slough
	Concrete		Grout		Solid Pipe
	Cover		Sand		Slotted Pipe

Soil Sample Type

	Thin Walled Tube		Disturbed		No Recovery
	Driven Spoon		Core (any type)		

Groundwater Symbols

	Piezometric elevation as determined by a piezometer installation
	Water levels measured in borings at the time and under the conditions noted



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**Bore Hole Logs
and
Laboratory Test Data**



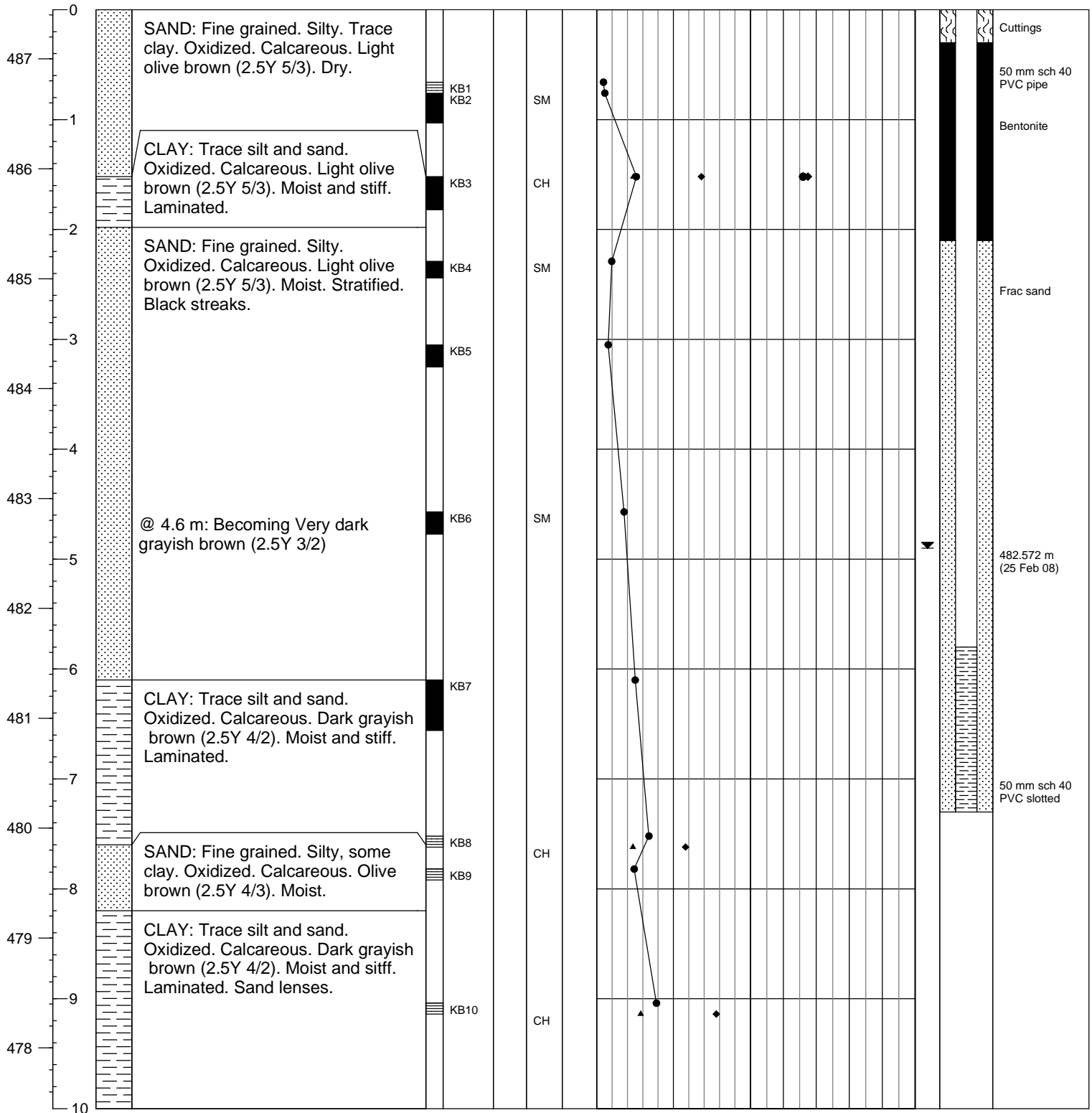
BORE HOLE LOG

 Bore Hole: **101**

Page: 1 of 2

Client: Neil Ketilson	Northing: 5,765,376.985 (UTM)	Date Drilled: 17 January 2007
Project: Casa Grande Subdivision	Easting: 388,679.686 (UTM)	Drill: Brat 22
Location: Grasswood, SK	Ground Elev.: 487.448 m (Geodetic)	Drilling Method: Solid Stem Auger
Project No.: S1607	Top Casing Elev.: 488.412 m	Logged by: KB

Elev (m) Depth (m)	Symbol	Soil Description	Sample Type No. SPT 'N' USC	% Sulphate	Moisture Content			Dry Density - kg/m ³			Piezometer Construction Detail
					Plastic Limit ▲	Natural Moisture ●	Liquid Limit ◆	Unconf. Shear ■	Pocket Pen. ●	Lab Vane ◆	

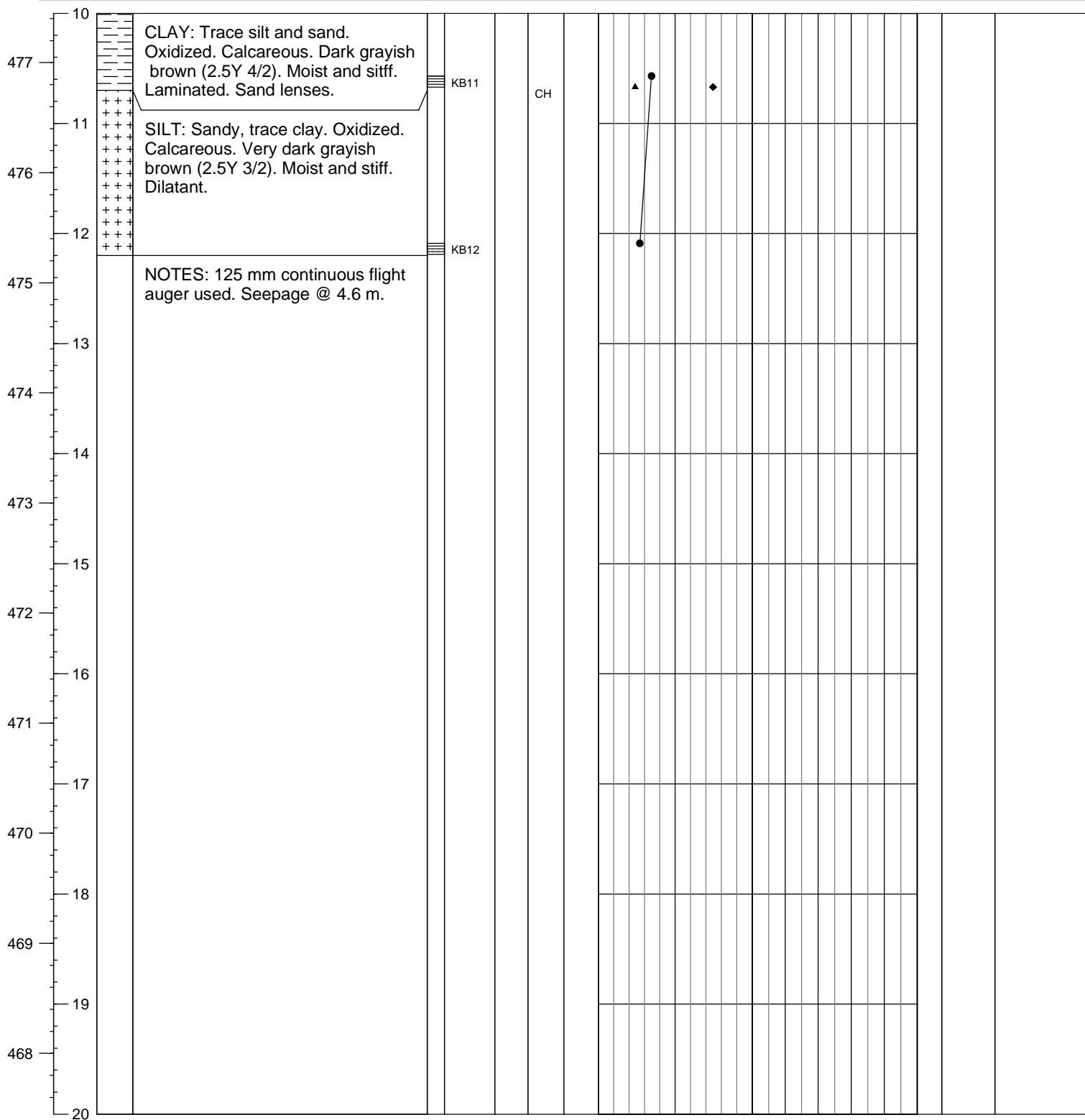




Client:	Neil Ketilson
Project:	Casa Grande Subdivision
Location:	Grasswood, SK
Project No.:	S1607

Northings:	5,765,376.985 (UTM)	Date Drilled:	17 January 2007
Easting:	388,679.686 (UTM)	Drill:	Brat 22
Ground Elev.:	487.448 m (Geodetic)	Drilling Method:	Solid Stem Auger
Top Casing Elev.:	488.412 m	Logged by:	KB

Elev (m)	Depth (m)	Symbol	Soil Description	Sample		USC	% Sulphate	Moisture Content			<div>▲ Dry Density - kg/m³</div> <div>1600 2000</div>				Piezometer Construction Detail
				Type	No.			SPT 'N'	Plastic Limit	percent Natural Moisture	Liquid Limit	Shear Unconf.	Strength Pocket Pen.	kPa Lab Vane	



Geotech BH m Elev CAL v03.tif



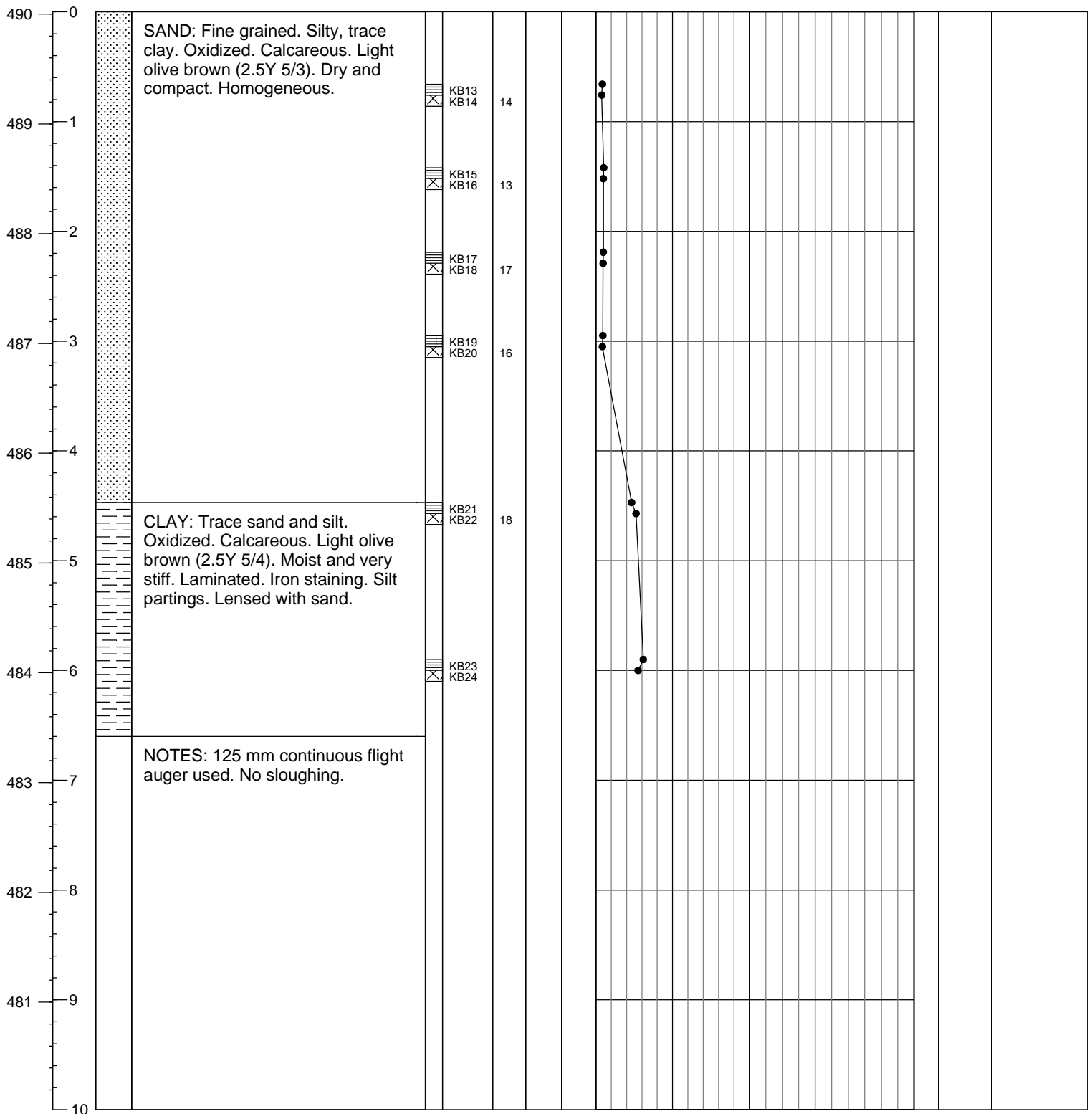
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BORE HOLE LOG

Bore Hole: 102
Page: 1 of 1

Client: Neil Ketilson Northing: 5,765,592.890 (UTM) Date Drilled: 17 January 2007
Project: Casa Grande Subdivision Easting: 388,709.567 (UTM) Drill: Brat 22
Location: Grasswood, SK Ground Elev.: 490.021 m (Geodetic) Drilling Method: Solid Stem Auger
Project No.: S1607 Top Casing Elev.: Logged by: KB

Elev (m) Depth (m)	Symbol	Soil Description	Sample		USC	% Sulphate	Moisture Content			Shear Strength - kPa Unconf. Pocket Pen. Lab Vane	Piezometer Construction Detail
			Type	No.			Plastic Limit ▲	Natural Moisture ●	Liquid Limit ◆		
							0	50	100	1600 2000	





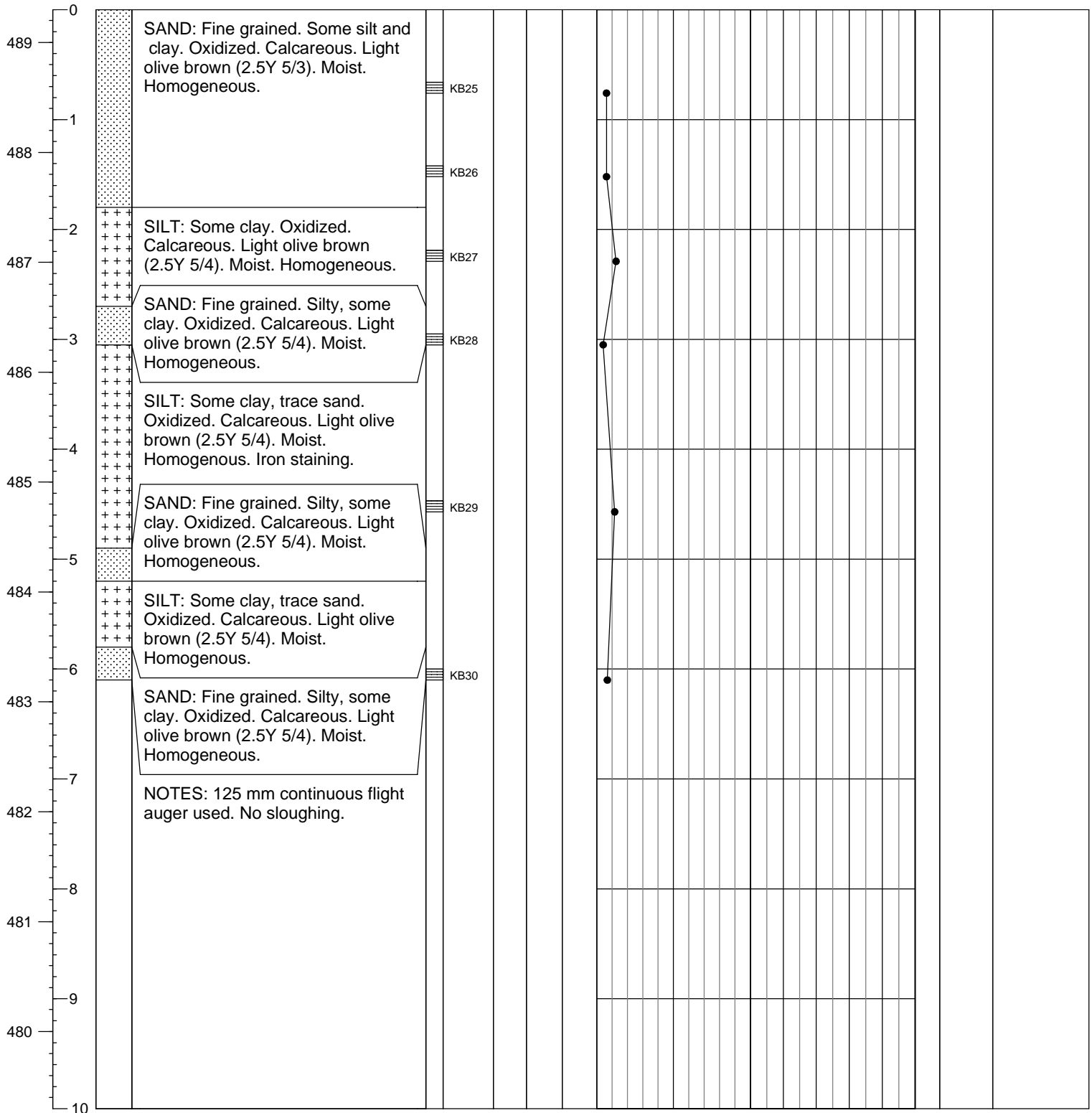
BORE HOLE LOG

 Bore Hole: **103**

Page: 1 of 1

Client: Neil Ketilson	Northing: 5,765,974.919 (UTM)	Date Drilled: 17 January 2007
Project: Casa Grande Subdivision	Easting: 388,747.079 (UTM)	Drill: Brat 22
Location: Grasswood, SK	Ground Elev.: 489.299 m (Geodetic)	Drilling Method: Solid Stem Auger
Project No.: S1607	Top Casing Elev.:	Logged by: KB

Elev (m) Depth (m)	Symbol	Soil Description	Sample				Moisture Content			Shear Strength - kPa			Piezometer Construction Detail
			Type	No.	SPT 'N	USC	% Sulphate	Plastic Limit	Natural Moisture	Liquid Limit	Unconf. Pocket Pen.	Lab Vane	

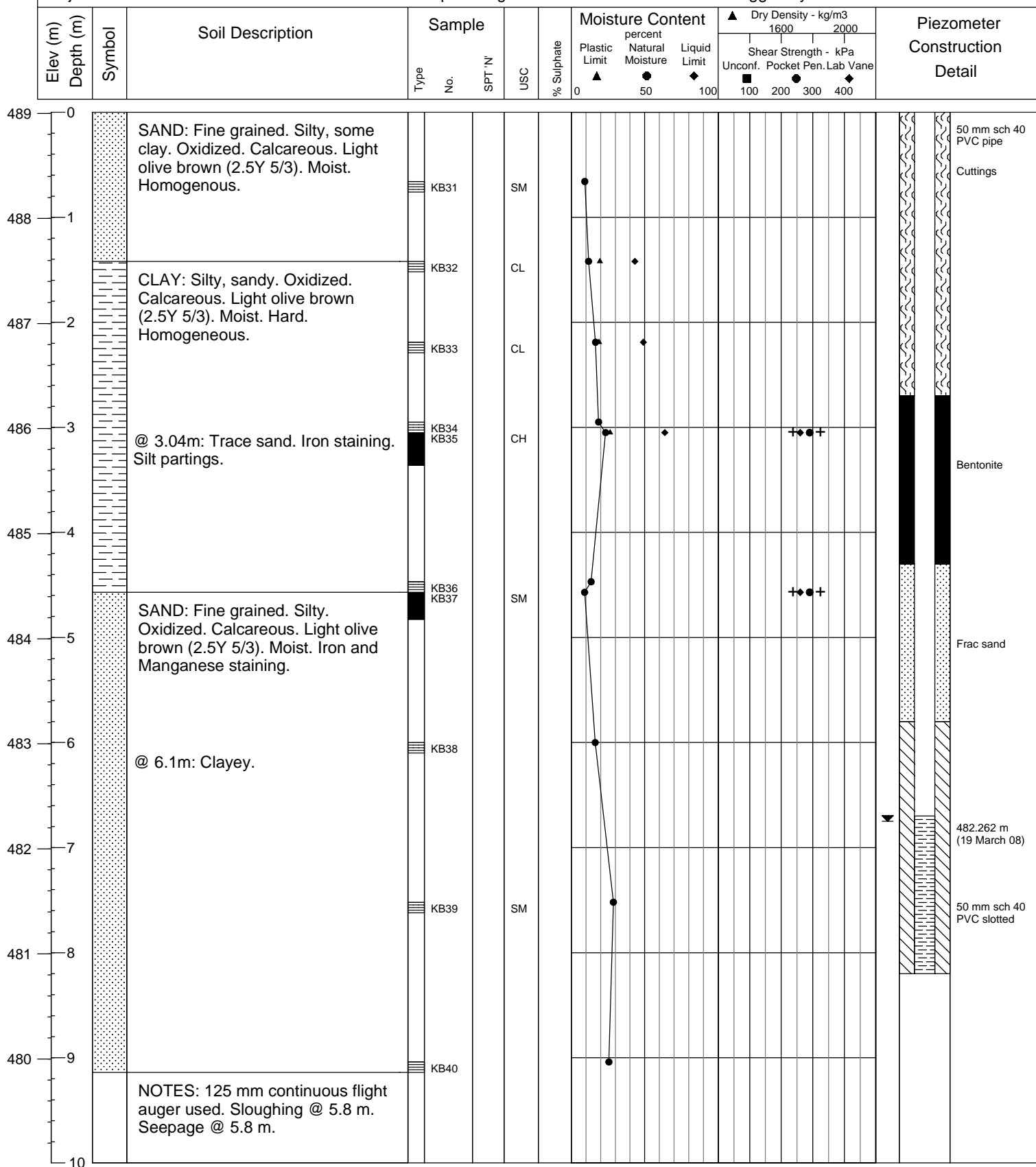




Client:	Neil Ketilson
Project:	Casa Grande Subdivision
Location:	Grasswood, SK
Project No.:	S1607

Northing:	5,766,116.545 (UTM)
Easting:	389,077.547 (UTM)
Ground Elev.:	489.012 m (Geodetic)
Top Casing Elev.:	489.896 m

Date Drilled: 17 January 2007
Drill: Brat 22
Drilling Method: Solid Stem Auger
Logged by: KB





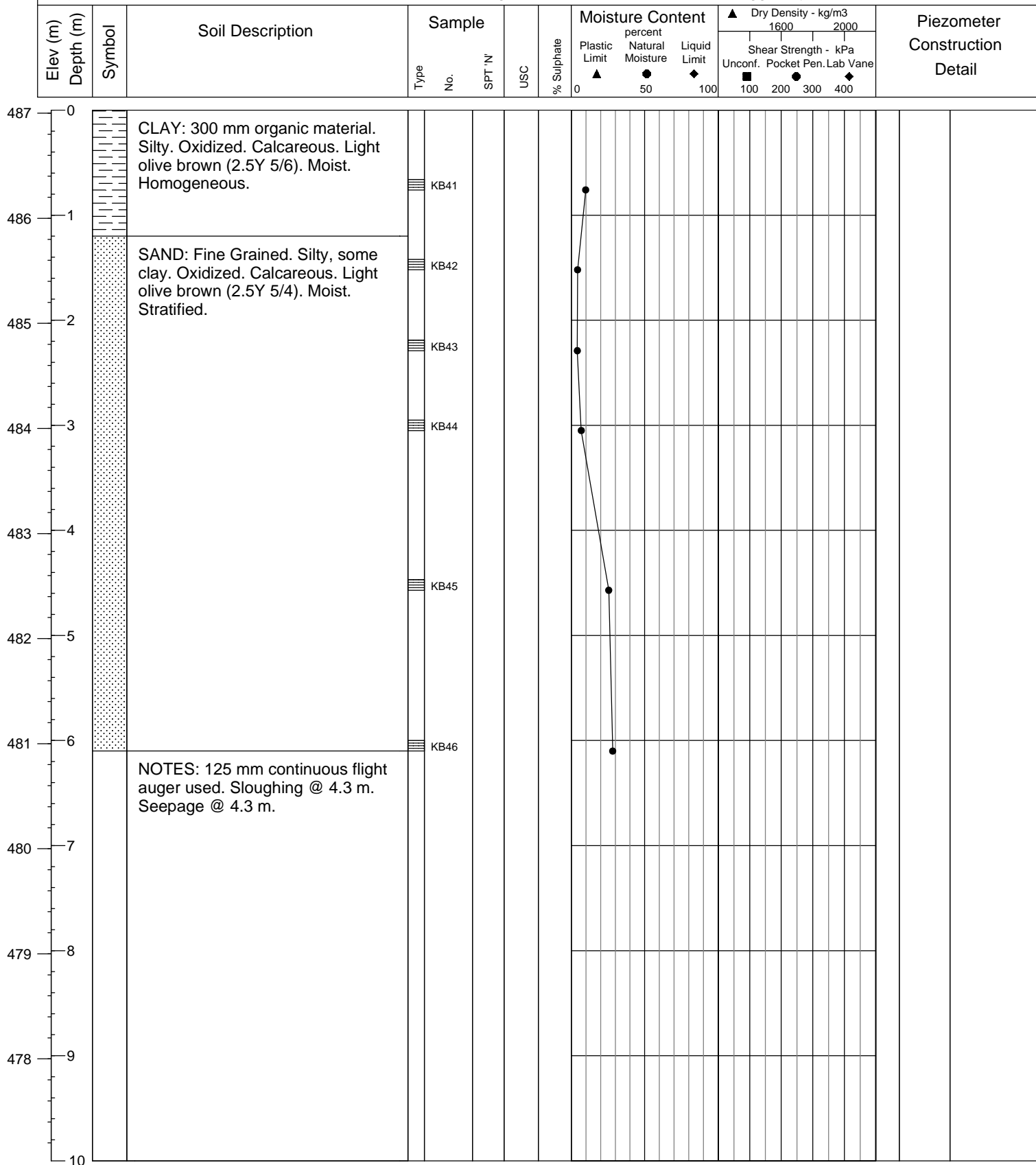
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BORE HOLE LOG

Bore Hole: **105**

Page: 1 of 1

Client: Neil Ketilson Northing: 5,765,861.154 (UTM) Date Drilled: 17 January 2007
 Project: Casa Grande Subdivision Easting: 389,077.457 (UTM) Drill: Brat 22
 Location: Grasswood, SK Ground Elev.: 487.030 m (Geodetic) Drilling Method: Solid Stem Auger
 Project No.: S1607 Top Casing Elev.: Logged by: KB





Client:	Neil Ketilson
Project:	Casa Grande Subdivision
Location:	Grasswood, SK
Project No.:	S1607

Northings:	5,765,600.569 (UTM)	Date Drilled:	17 January 2007
Easting:	389,053.995 (UTM)	Drill:	Brat 22
Ground Elev.:	489.115 m (Geodetic)	Drilling Method:	Solid Stem Auger
Top Casing Elev.:		Logged by:	KB

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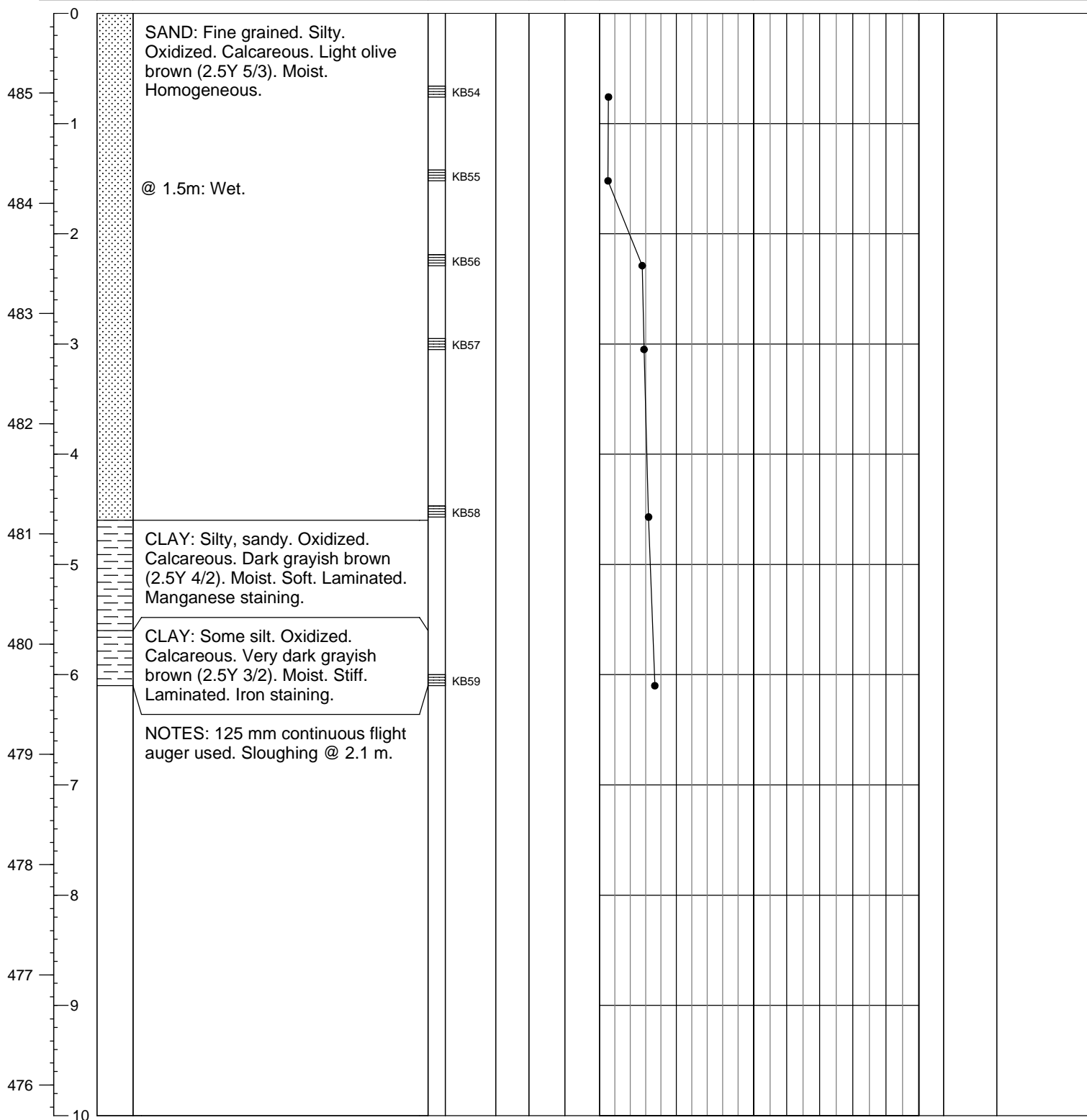
BORE HOLE LOG

 Bore Hole: **107**

Page: 1 of 1

Client:	Neil Ketilson	Northing:	5,765,367.001 (UTM)	Date Drilled:	17 January 2007
Project:	Casa Grande Subdivision	Easting:	389,086.446 (UTM)	Drill:	Brat 22
Location:	Grasswood, SK	Ground Elev.:	485.723 m (Geodetic)	Drilling Method:	Solid Stem Auger
Project No.:	S1607	Top Casing Elev.:		Logged by:	KB

Elev (m) Depth (m)	Symbol	Soil Description	Sample				Moisture Content			Dry Density - kg/m ³		Piezometer Construction Detail
			Type	No.	SPT 'N	USC	% Sulphate	Plastic Limit	Natural Moisture	Liquid Limit	Shear Strength - kPa	
								▲	●	◆	Unconf. Pocket Pen. Lab Vane	
								0	50	100	100 200 300 400	





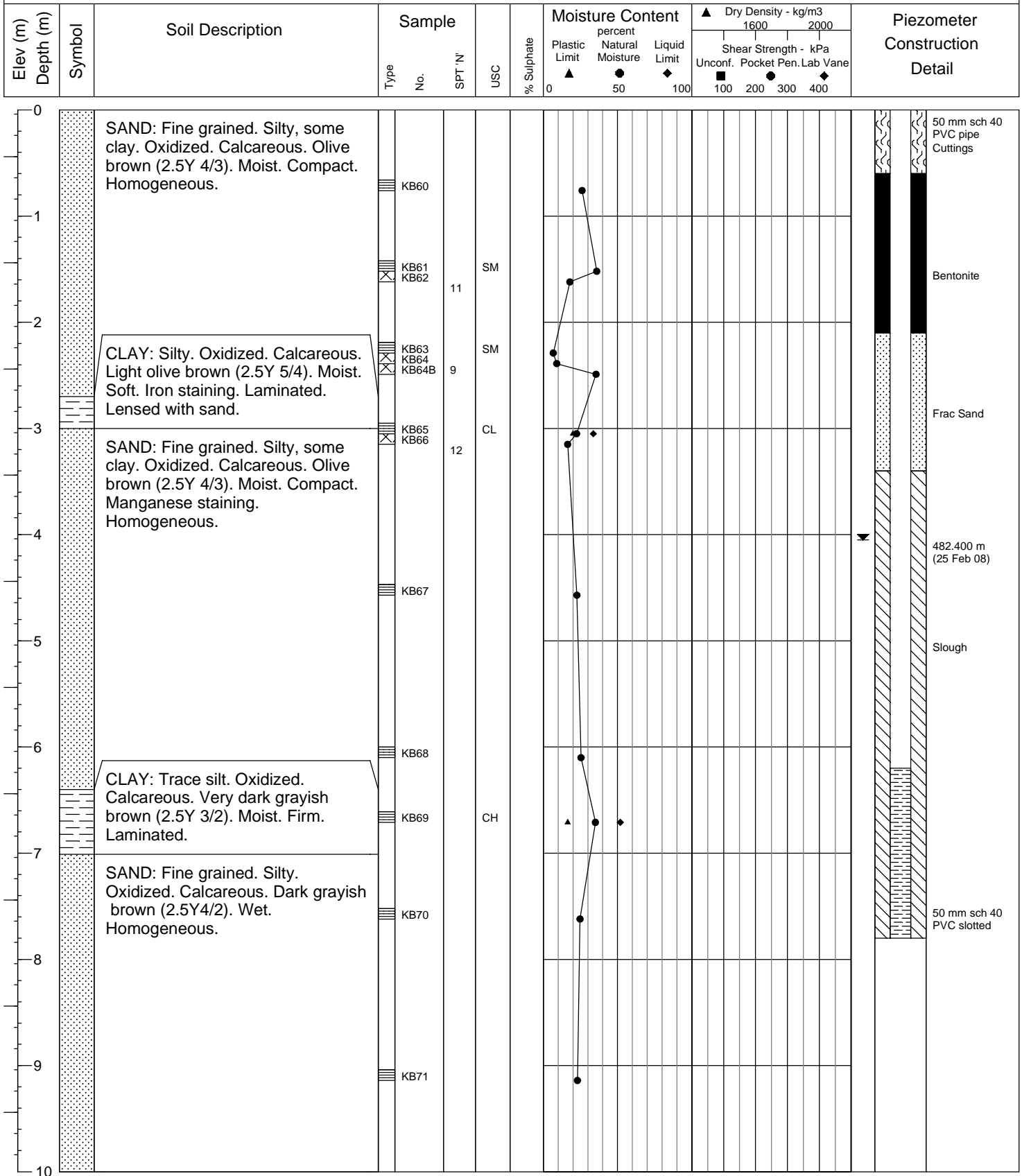
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BORE HOLE LOG

Bore Hole: 108

Page: 1 of 2

Client: Neil Ketilson	Northing: 5,765,324.170 (UTM)	Date Drilled: 17 January 2007
Project: Casa Grande Subdivision	Easting: 389,461.073 (UTM)	Drill: Brat 22
Location: Grasswood, SK	Ground Elev.: 486.440 m (Geodetic)	Drilling Method: Solid Stem Auger
Project No.: S1607	Top Casing Elev.: 487.390 m	Logged by: KB

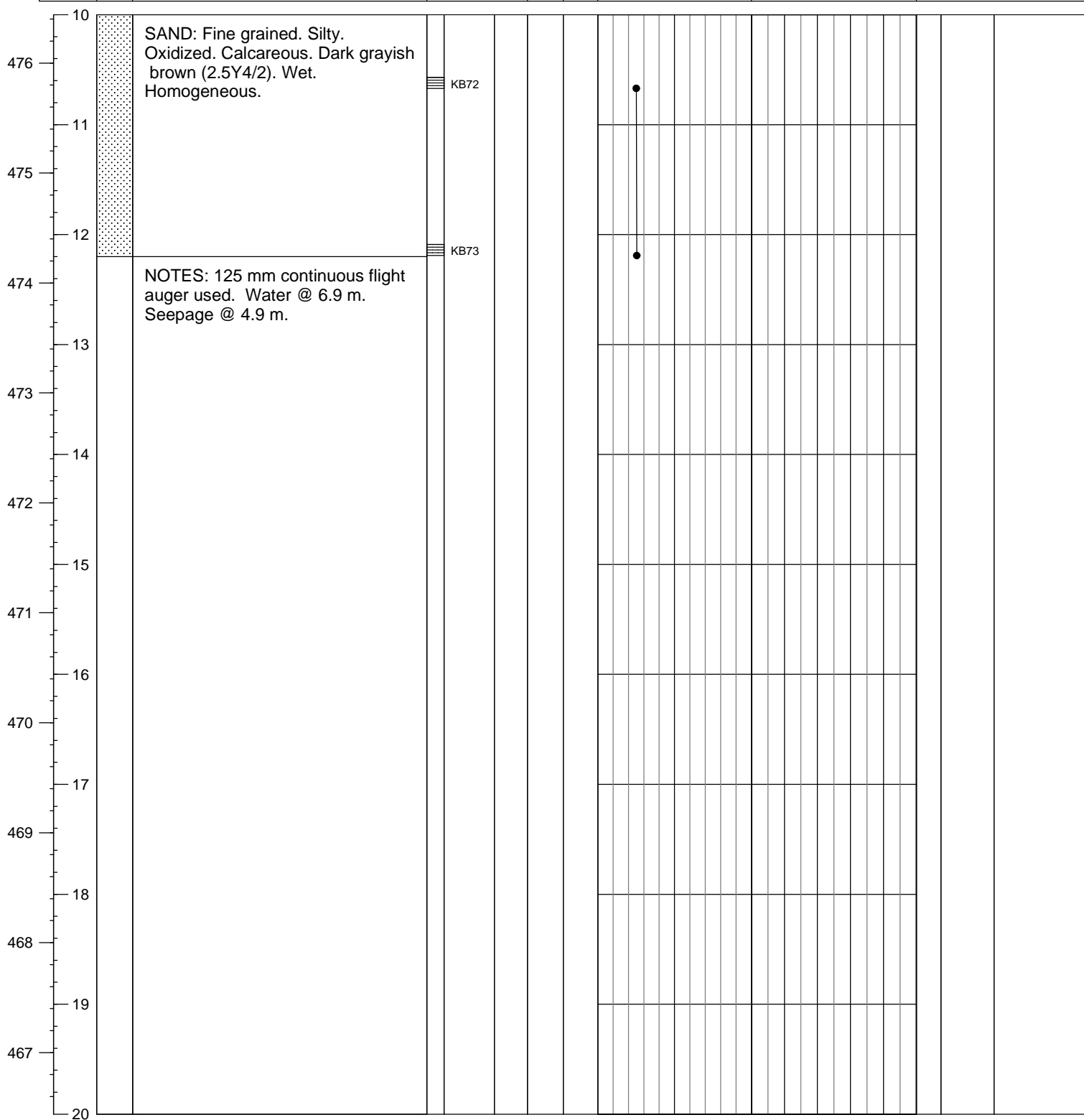




Client:	Neil Ketilson
Project:	Casa Grande Subdivision
Location:	Grasswood, SK
Project No.:	S1607

Northings:	5,765,324.170 (UTM)	Date Drilled:	17 January 2007
Easting:	389,461.073 (UTM)	Drill:	Brat 22
Ground Elev.:	486.440 m (Geodetic)	Drilling Method:	Solid Stem Auger
Top Casing Elev.:	487.390 m	Logged by:	KB

Elev (m)	Depth (m)	Symbol	Soil Description	Sample		USC	% Sulphate	Moisture Content			<div>▲ Dry Density - kg/m3</div> <div>1600 2000</div>				Piezometer Construction Detail
				Type	No.			SPT 'N'	Plastic Limit	percent Natural Moisture	Liquid Limit	Shear Unconf.	Strength Pocket Pen.	kPa Lab Vane	





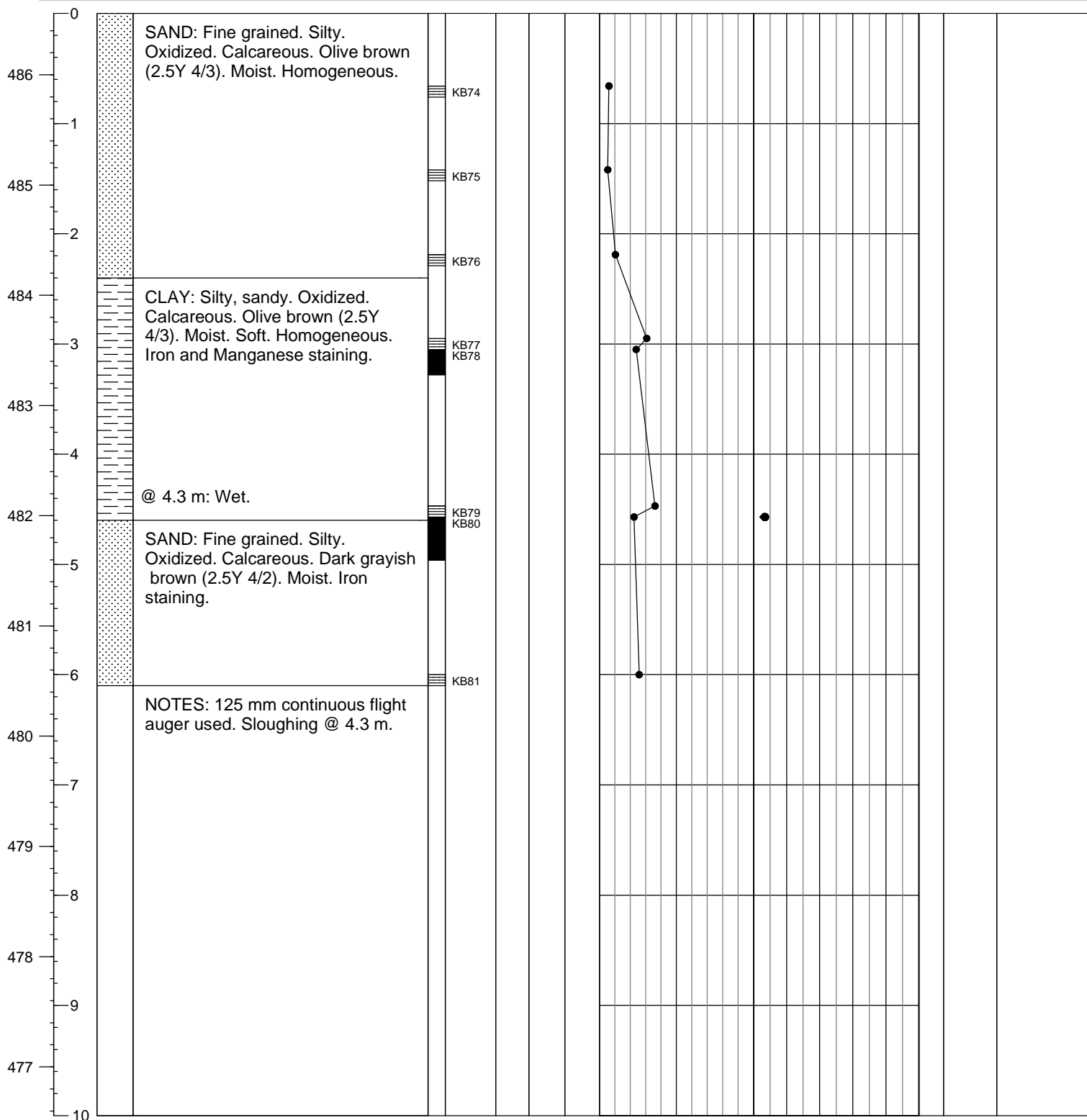
BORE HOLE LOG

 Bore Hole: **109**

Page: 1 of 1

Client: Neil Ketilson	Northing: 5,765,594.201 (UTM)	Date Drilled: 17 January 2007
Project: Casa Grande Subdivision	Easting: 389,458.870 (UTM)	Drill: Brat 22
Location: Grasswood, SK	Ground Elev.: 486.558 m (Geodetic)	Drilling Method: Solid Stem Auger
Project No.: S1607	Top Casing Elev.:	Logged by: KB

Elev (m) Depth (m)	Symbol	Soil Description	Sample				Moisture Content			Shear Strength - kPa			Piezometer Construction Detail
			Type	No.	SPT 'N	USC	% Sulphate	Plastic Limit	Natural Moisture	Liquid Limit	Unconf. Pocket Pen.	Lab Vane	





Client:	Neil Ketilson
Project:	Casa Grande Subdivision
Location:	Grasswood, SK
Project No.:	S1607

Northings:	5,765,865.177 (UTM)	Date Drilled:	17 January 2008
Easting:	389,397.072 (UTM)	Drill:	Brat 22
Ground Elev.:	487.043 m (Geodetic)	Drilling Method:	Solid Stem Auger
Top Casing Elev.:		Logged by:	KB

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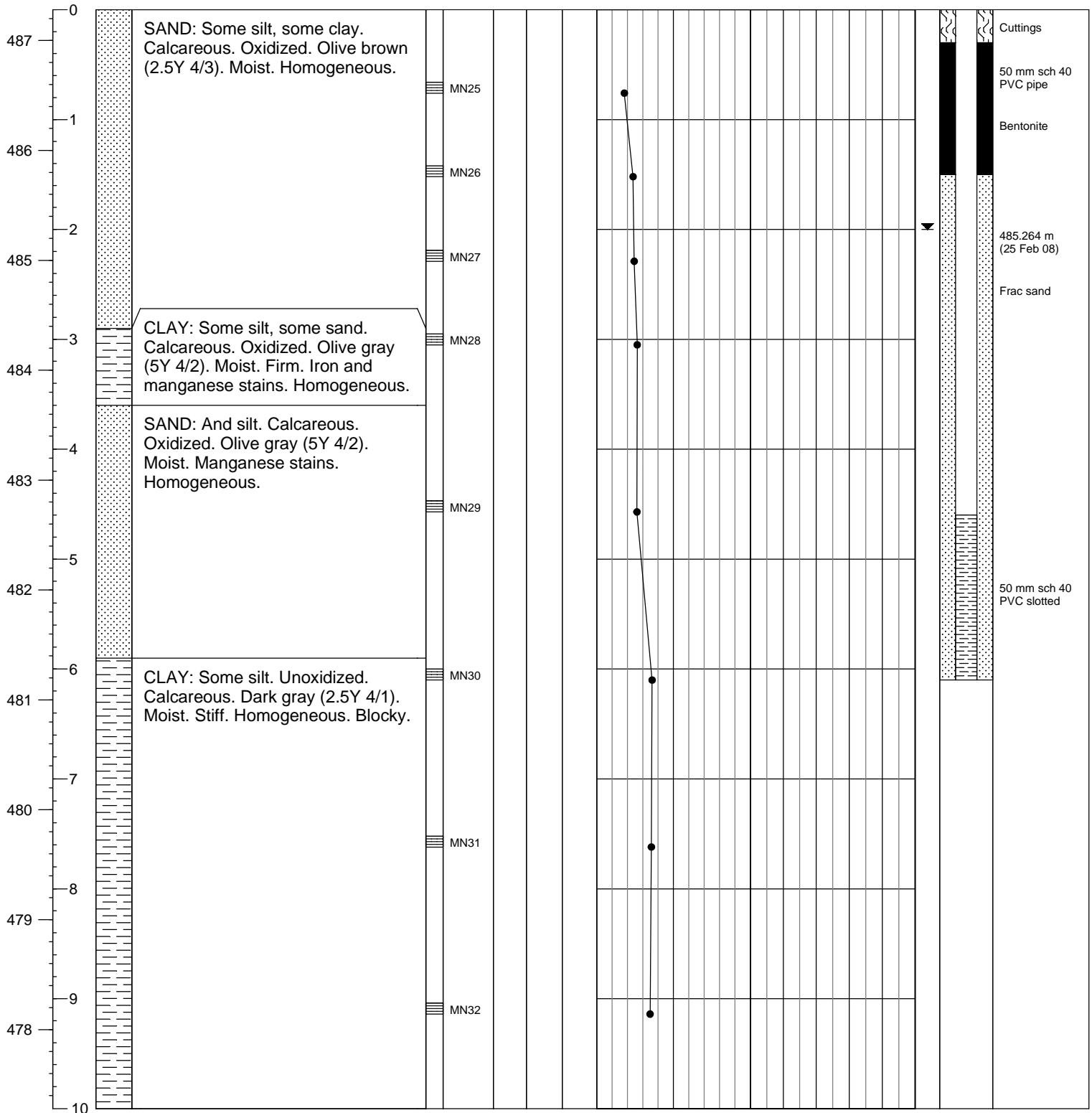
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BORE HOLE LOG

Bore Hole: 111
Page: 1 of 2

Client: Neil Ketilson	Northing: 5,766,906.065 (UTM)	Date Drilled: 08 February 2008
Project: Casa Grande Subdivision	Easting: 389,475.333 (UTM)	Drill: Brat 22
Location: Grasswood, SK	Ground Elev.: 487.281 m (Geodetic)	Drilling Method: Solid Stem Auger
Project No.: S1607	Top Casing Elev.: 488.094 m	Logged by: MN

Elev (m) Depth (m)	Symbol	Soil Description	Sample Type No. SPT 'N' USC	% Sulphate	Moisture Content			Shear Strength - kPa			Piezometer Construction Detail
					Plastic Limit ▲	Natural Moisture ●	Liquid Limit ◆	Unconf. Pocket Pen. Lab Vane	Unconf. Pocket Pen. Lab Vane	Unconf. Pocket Pen. Lab Vane	



Geotech BH m Elev CAL v03.tif



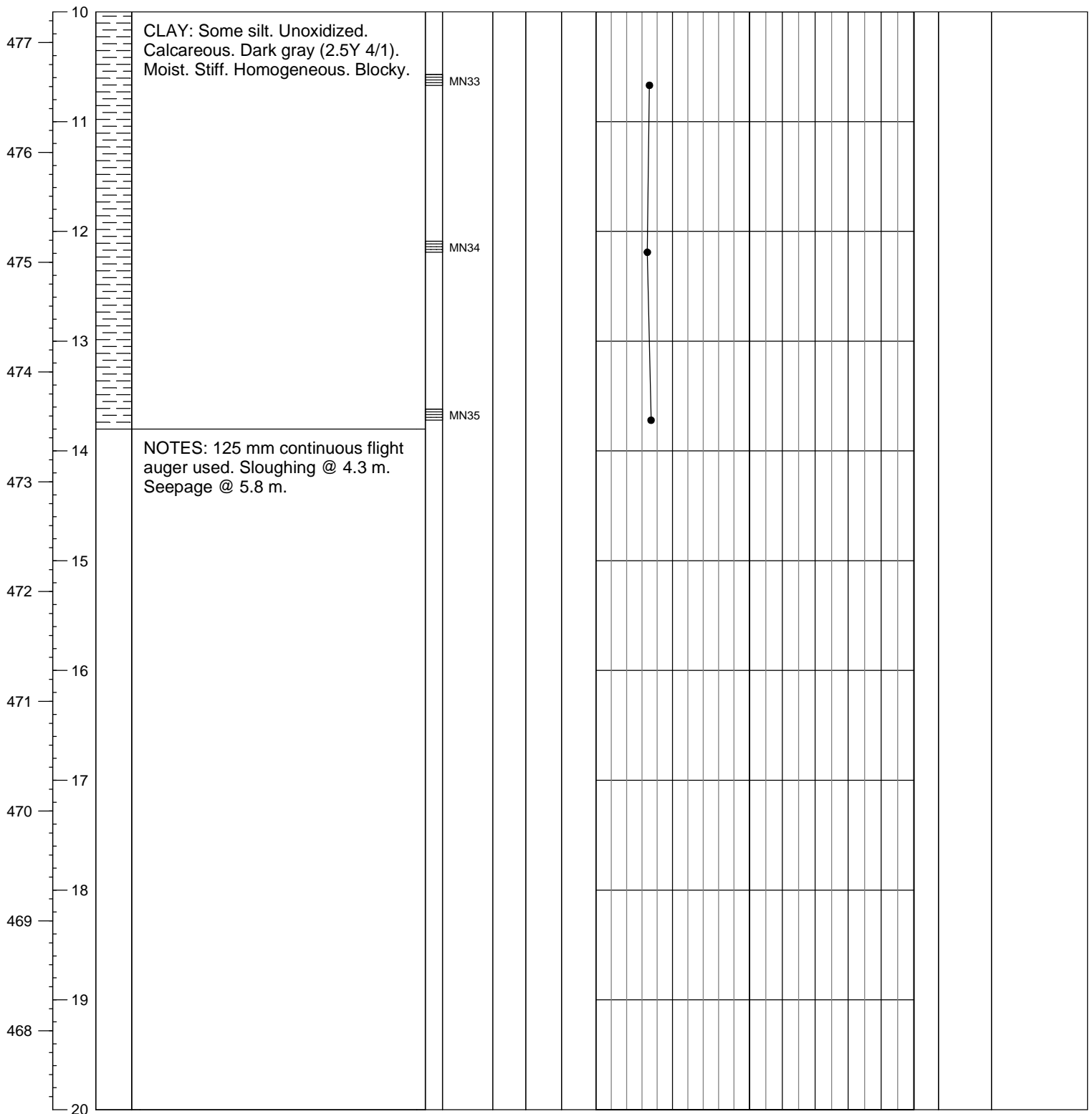
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BORE HOLE LOG

Bore Hole: 111
Page: 2 of 2

Client: Neil Ketilson Northing: 5,766,906.065 (UTM) Date Drilled: 08 February 2008
Project: Casa Grande Subdivision Easting: 389,475.333 (UTM) Drill: Brat 22
Location: Grasswood, SK Ground Elev.: 487.281 m (Geodetic) Drilling Method: Solid Stem Auger
Project No.: S1607 Top Casing Elev.: 488.094 m Logged by: MN

Elev (m) Depth (m)	Symbol	Soil Description	Sample			USC	% Sulphate	Moisture Content			▲ Dry Density - kg/m3 1600 2000			Piezometer Construction Detail
			Type	No.	SPT 'N'			Plastic Limit	percent Natural Moisture	Liquid Limit	Shear Strength - kPa Unconf. Pocket Pen. Lab Vane			



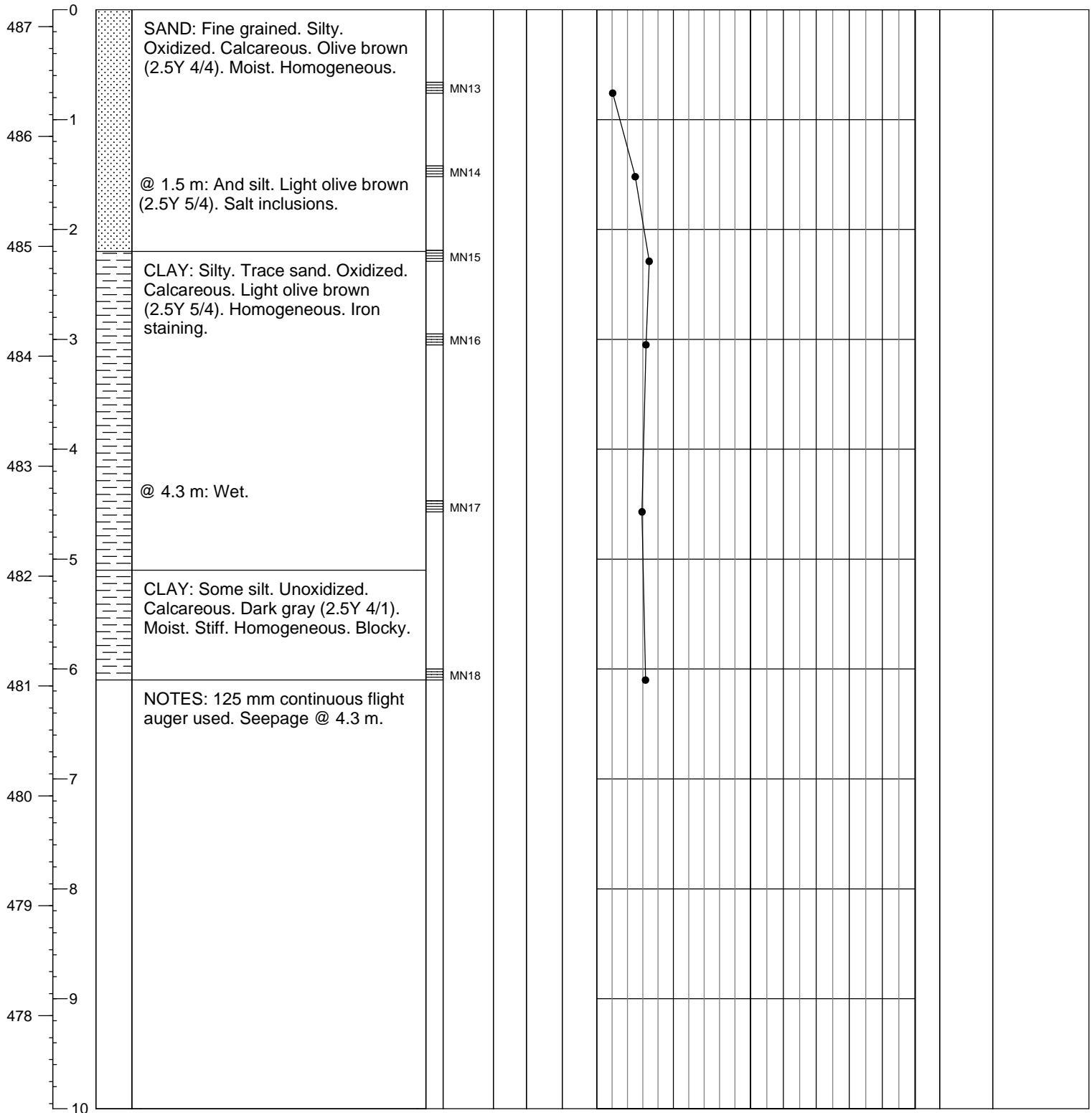


BORE HOLE LOG

 Bore Hole: **112**
 Page: 1 of 1

Client: Neil Ketilson	Northing: 5,766,912.968 (UTM)	Date Drilled: 08 February 2008
Project: Casa Grande Subdivision	Easting: 389,067.950 (UTM)	Drill: Brat 22
Location: Grasswood, SK	Ground Elev.: 487.154 m (Geodetic)	Drilling Method: Solid Stem Auger
Project No.: S1607	Top Casing Elev.:	Logged by: MN

Elev (m) Depth (m)	Symbol	Soil Description	Sample		USC	% Sulphate	Moisture Content			▲ Dry Density - kg/m3 1600 2000			Piezometer Construction Detail
			Type	No.			SPT N	Plastic Limit	percent Natural Moisture	Liquid Limit	Shear Strength - kPa Unconf. Pocket Pen. Lab Vane		



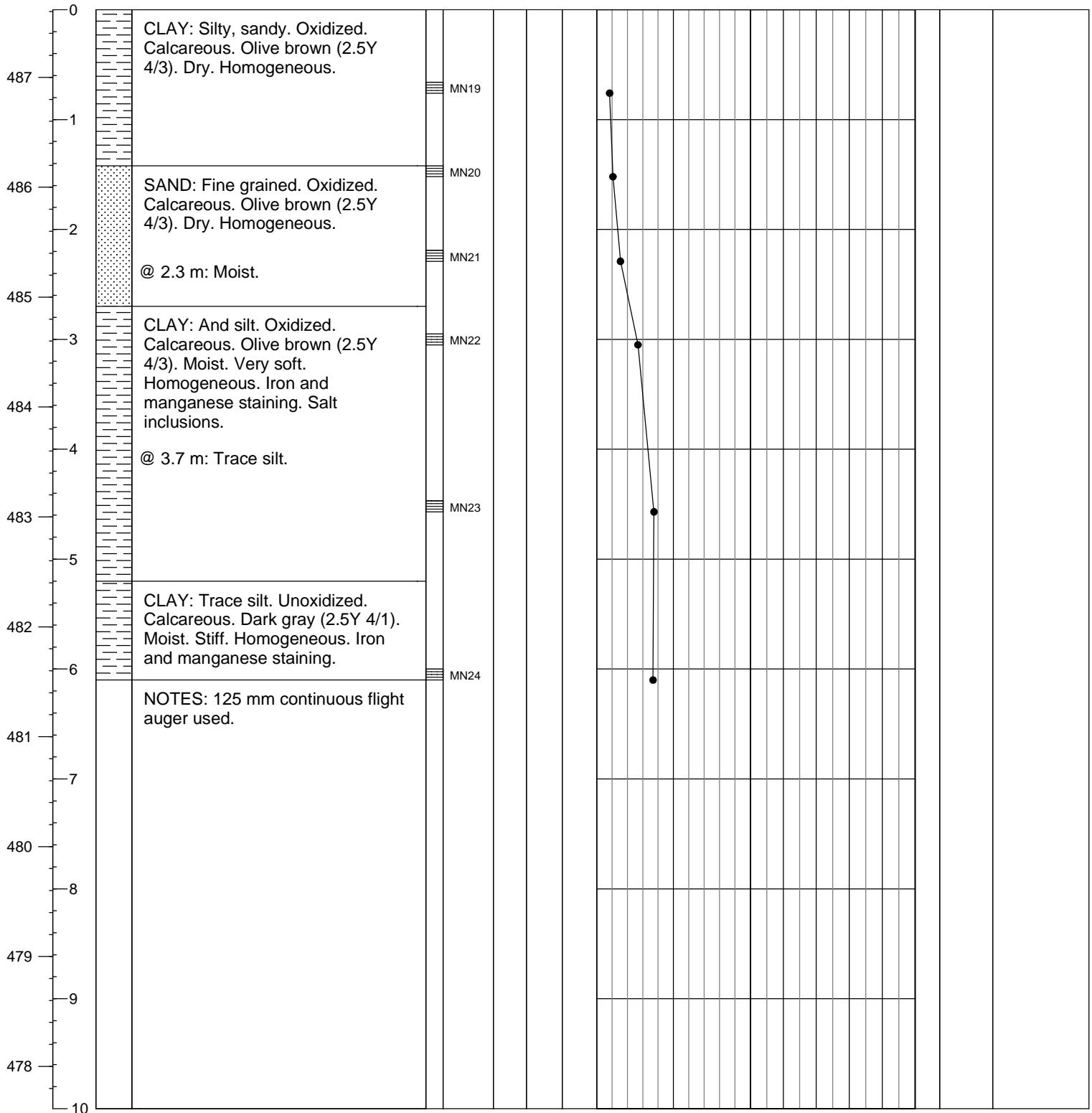


BORE HOLE LOG

 Bore Hole: **113**
 Page: 1 of 1

Client: Neil Ketilson	Northing: 5,766,642.442 (UTM)	Date Drilled: 08 February 2008
Project: Casa Grande Subdivision	Easting: 389,078.575 (UTM)	Drill: Brat 22
Location: Grasswood, SK	Ground Elev.: 487.616 m (Geodetic)	Drilling Method: Solid Stem Auger
Project No.: S1607	Top Casing Elev.:	Logged by: MN

Elev (m) Depth (m)	Symbol	Soil Description	Sample		USC	% Sulphate	Moisture Content			▲ Dry Density - kg/m3 1600 2000			Piezometer Construction Detail
			Type	No.			SPT N'	Plastic Limit	percent Natural Moisture	Liquid Limit	Shear Strength - kPa Unconf. Pocket Pen. Lab Vane		





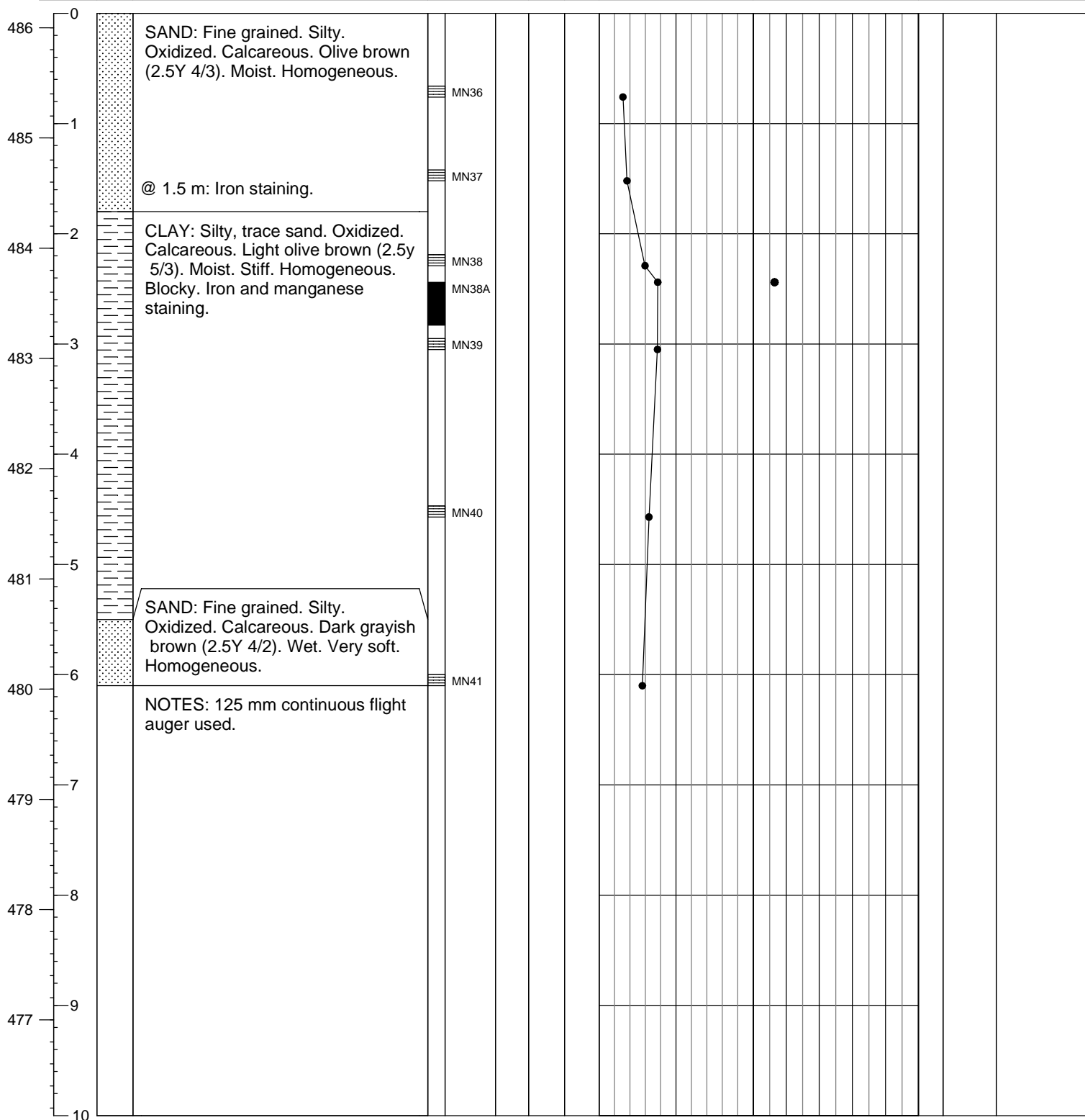
BORE HOLE LOG

 Bore Hole: **114**

Page: 1 of 1

Client: Neil Ketilson	Northing: 5,766,644.763 (UTM)	Date Drilled: 08 February 2008
Project: Casa Grande Subdivision	Easting: 389,462.535 (UTM)	Drill: Brat 22
Location: Grasswood, SK	Ground Elev.: 486.131m (Geodetic)	Drilling Method: Solid Stem Auger
Project No.: S1607	Top Casing Elev.:	Logged by: MN

Elev (m) Depth (m)	Symbol	Soil Description	Sample				Moisture Content			Shear Strength - kPa			Piezometer Construction Detail
			Type	No.	SPT 'N	USC	% Sulphate	Plastic Limit	Natural Moisture	Liquid Limit	Unconf. Pocket Pen.	Lab Vane	





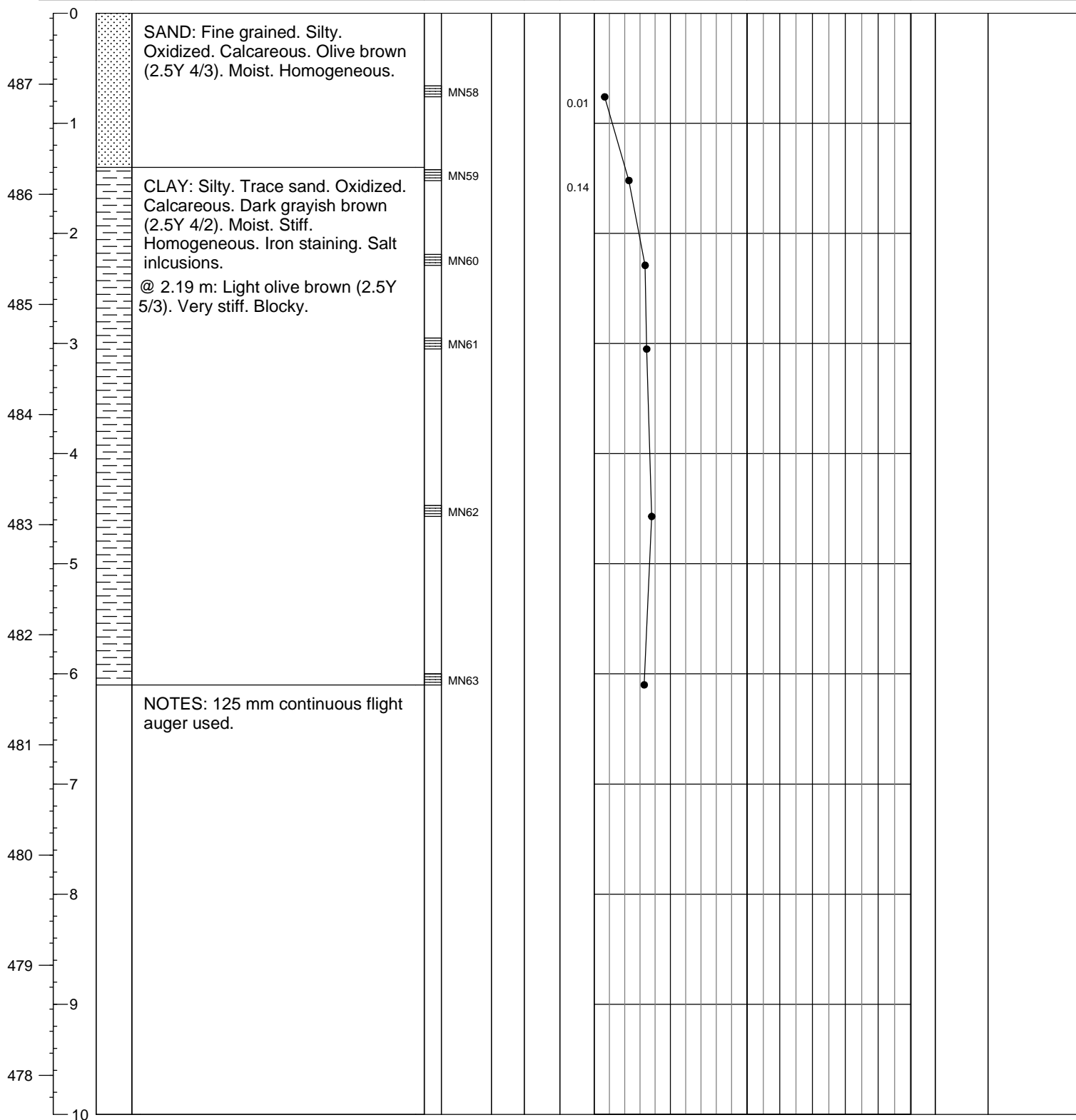
BORE HOLE LOG

 Bore Hole: **115**

Page: 1 of 1

Client: Neil Ketilson	Northing: 5,766,653.031 (UTM)	Date Drilled: 08 February 2008
Project: Casa Grande Subdivision	Easting: 388,734.367 m (UTM)	Drill: Brat 22
Location: Grasswood, SK	Ground Elev.: 487.645 m (Geodetic)	Drilling Method: Solid Stem Auger
Project No.: S1607	Top Casing Elev.:	Logged by: MN

Elev (m) Depth (m)	Symbol	Soil Description	Sample				Moisture Content			Shear Strength - kPa			Piezometer Construction Detail
			Type	No.	SPT 'N	USC	% Sulphate	Plastic Limit	Natural Moisture	Liquid Limit	Unconf. Pocket Pen.	Lab Vane	

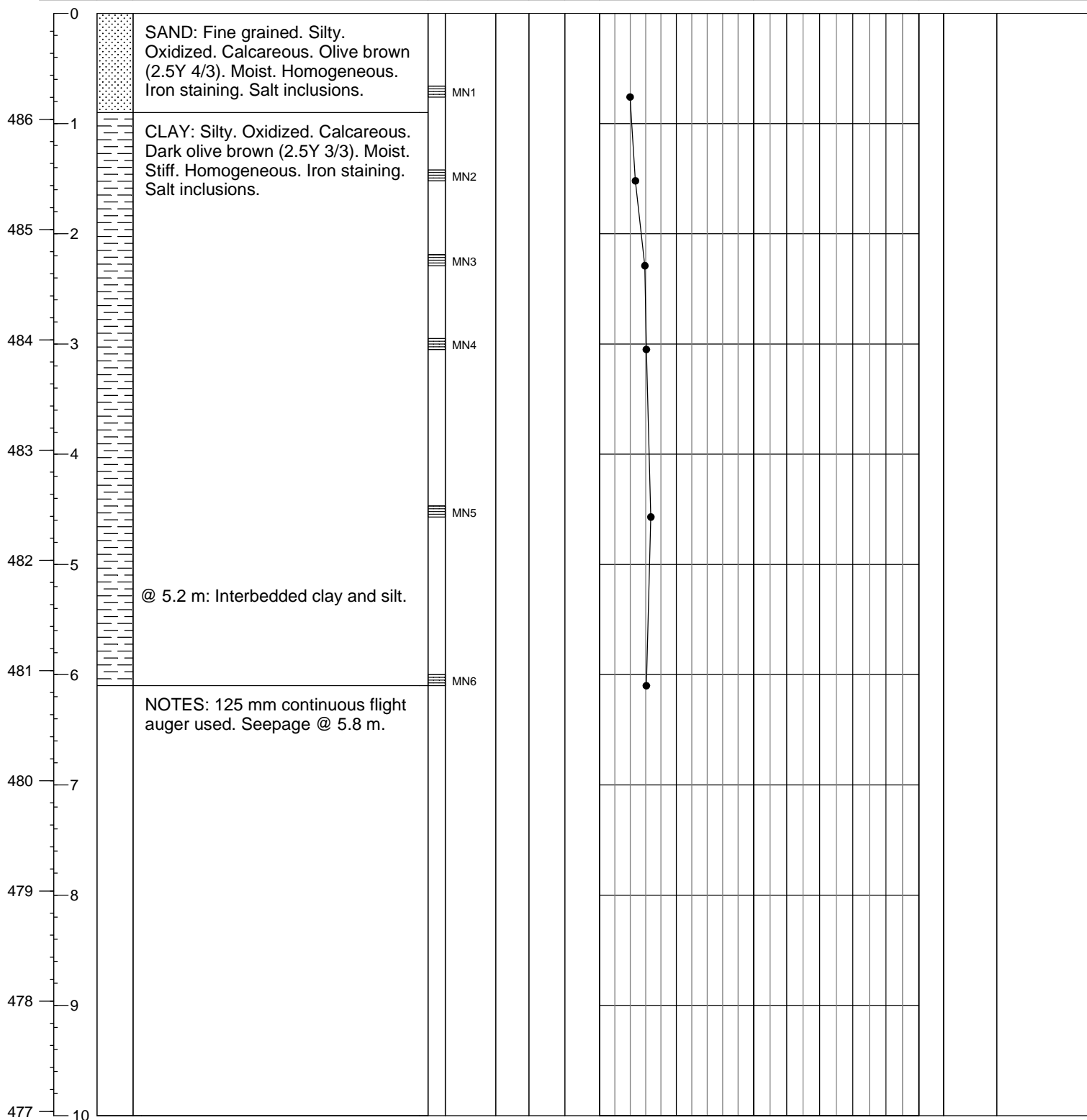




Client:	Neil Ketilson
Project:	Casa Grande Subdivision
Location:	Grasswood, SK
Project No.:	S1607

Northings:	5,765,643.118 (UTM)	Date Drilled:	08 February 2008
Easting:	389,291.604 (UTM)	Drill:	Brat 22
Ground Elev.:	486.963 m (Geodetic)	Drilling Method:	Solid Stem Auger
Top Casing Elev.:		Logged by:	MN

Elev (m)	Depth (m)	Symbol	Soil Description	Sample		USC	% Sulphate	Moisture Content			<div>▲ Dry Density - kg/m³</div> <div>1600 2000</div>				Piezometer Construction Detail
				Type	No.			SPT 'N'	Plastic Limit	percent Natural Moisture	Liquid Limit	Shear Unconf.	Strength Pocket Pen.	kPa Lab Vane	

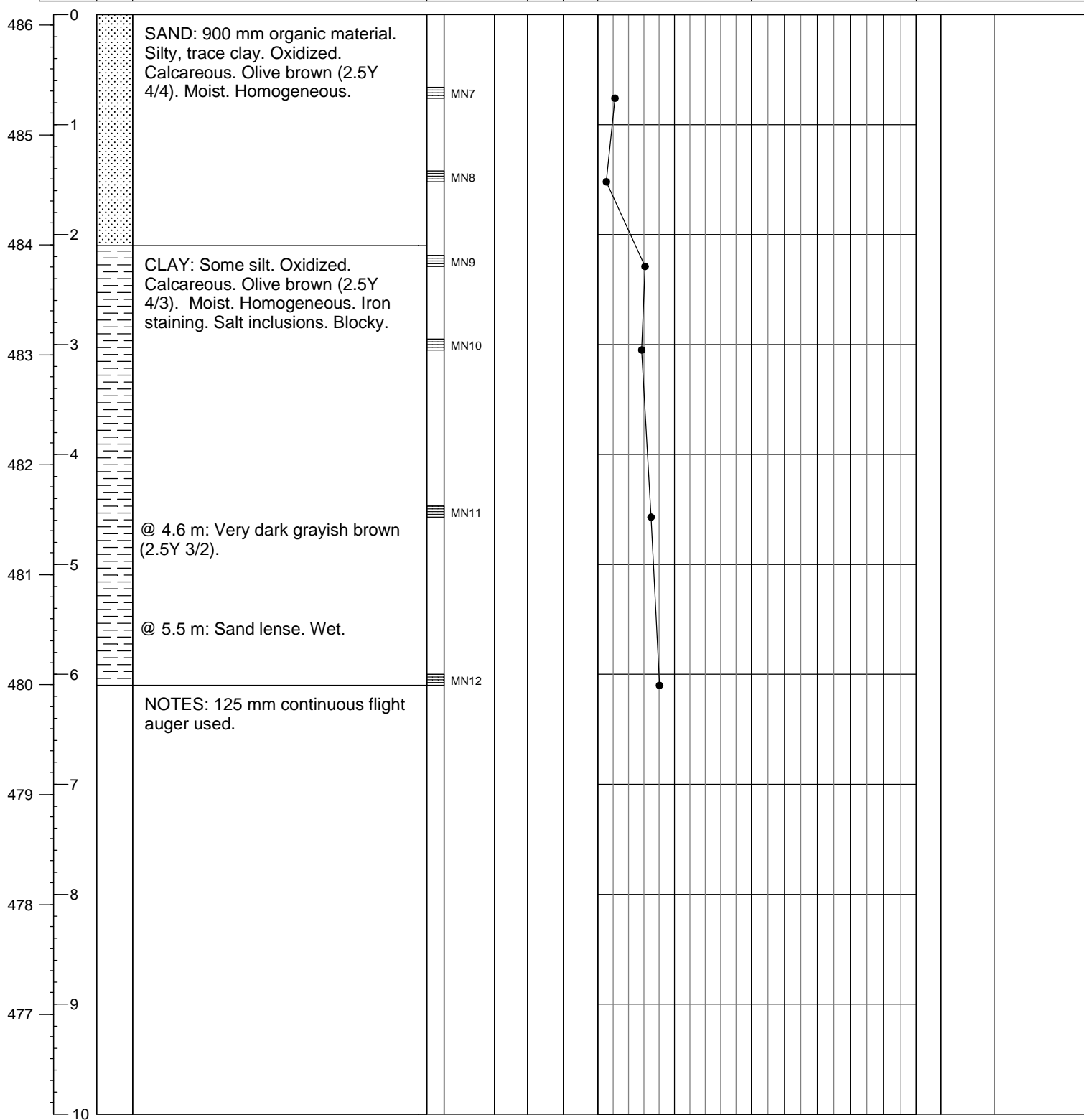




Client:	Neil Ketilson
Project:	Casa Grande Subdivision
Location:	Grasswood, SK
Project No.:	S1607

Northing:	5,765,569.269 (UTM)	Date Drilled:	08 February 2008
Easting:	388,792.867 (UTM)	Drill:	Brat 22
Ground Elev.:	486.094 m (Geodetic)	Drilling Method:	Solid Stem Auger
Top Casing Elev.:		Logged by:	MN

Elev (m)	Depth (m)	Symbol	Soil Description	Sample		USC	% Sulphate	Moisture Content			<div>▲ Dry Density - kg/m³</div> <div>1600 2000</div>				Piezometer Construction Detail
				Type	No.			SPT 'N'	Plastic Limit	percent Natural Moisture	Liquid Limit	Shear Unconf.	Strength Pocket Pen.	kPa Lab Vane	





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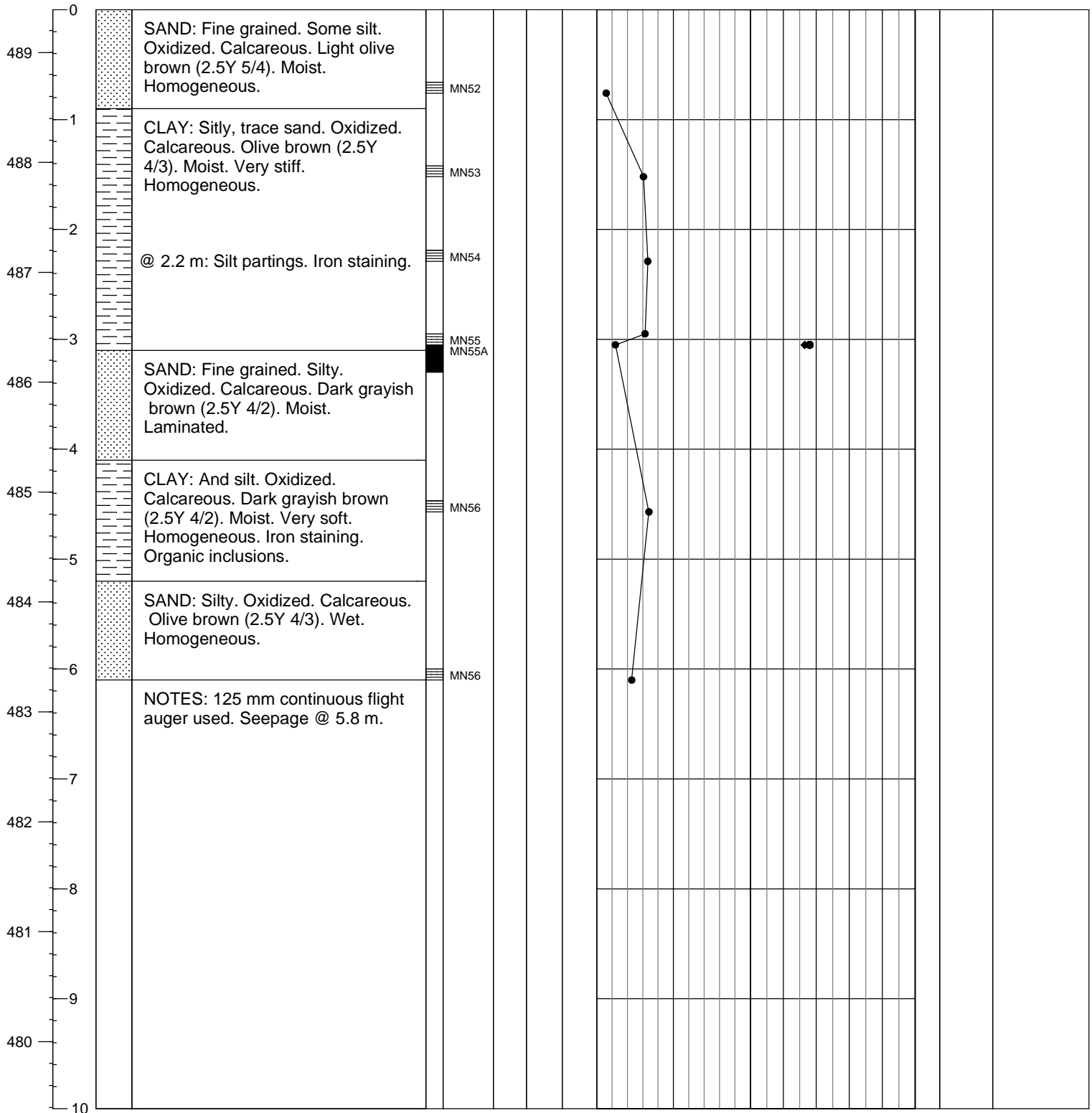
BORE HOLE LOG

Bore Hole: 118

Page: 1 of 1

Client: Neil Ketilson	Northing: 5,766,549.263 (UTM)	Date Drilled: 08 February 2008
Project: Casa Grande Subdivision	Easting: 388,851.487 (UTM)	Drill: Brat 22
Location: Grasswood, SK	Ground Elev.: 489.390 m (Geodetic)	Drilling Method: Solid Stem Auger
Project No.: S1607	Top Casing Elev.:	Logged by: MN

Elev (m) Depth (m)	Symbol	Soil Description	Sample		USC	% Sulphate	Moisture Content			▲ Dry Density - kg/m3 1600 2000			Piezometer Construction Detail
			Type	No.			SPT 'N'	Plastic Limit	percent Natural Moisture	Liquid Limit	Shear Strength - kPa Unconf. Pocket Pen. Lab Vane		





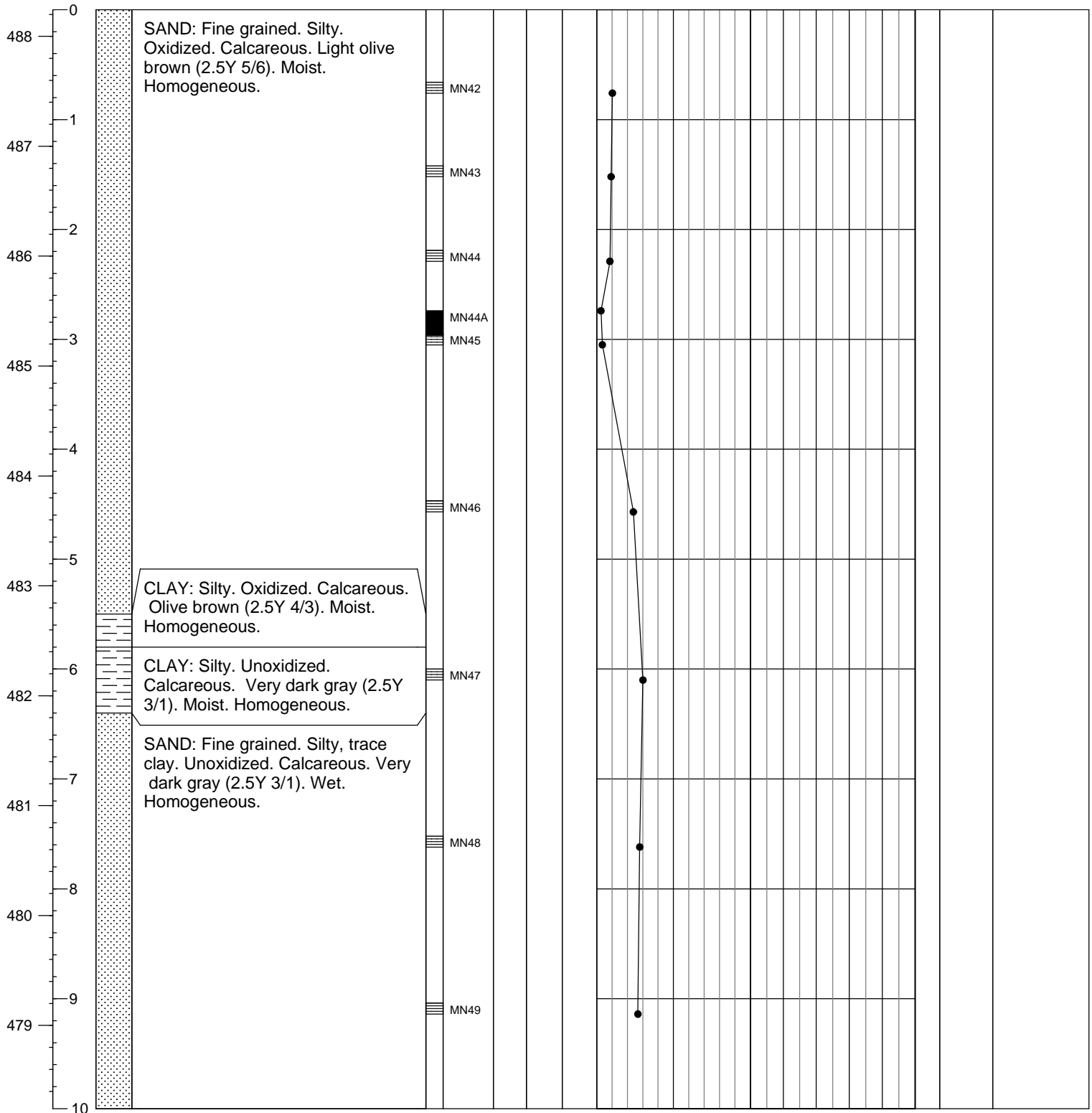
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BORE HOLE LOG

Bore Hole: 119
Page: 1 of 2

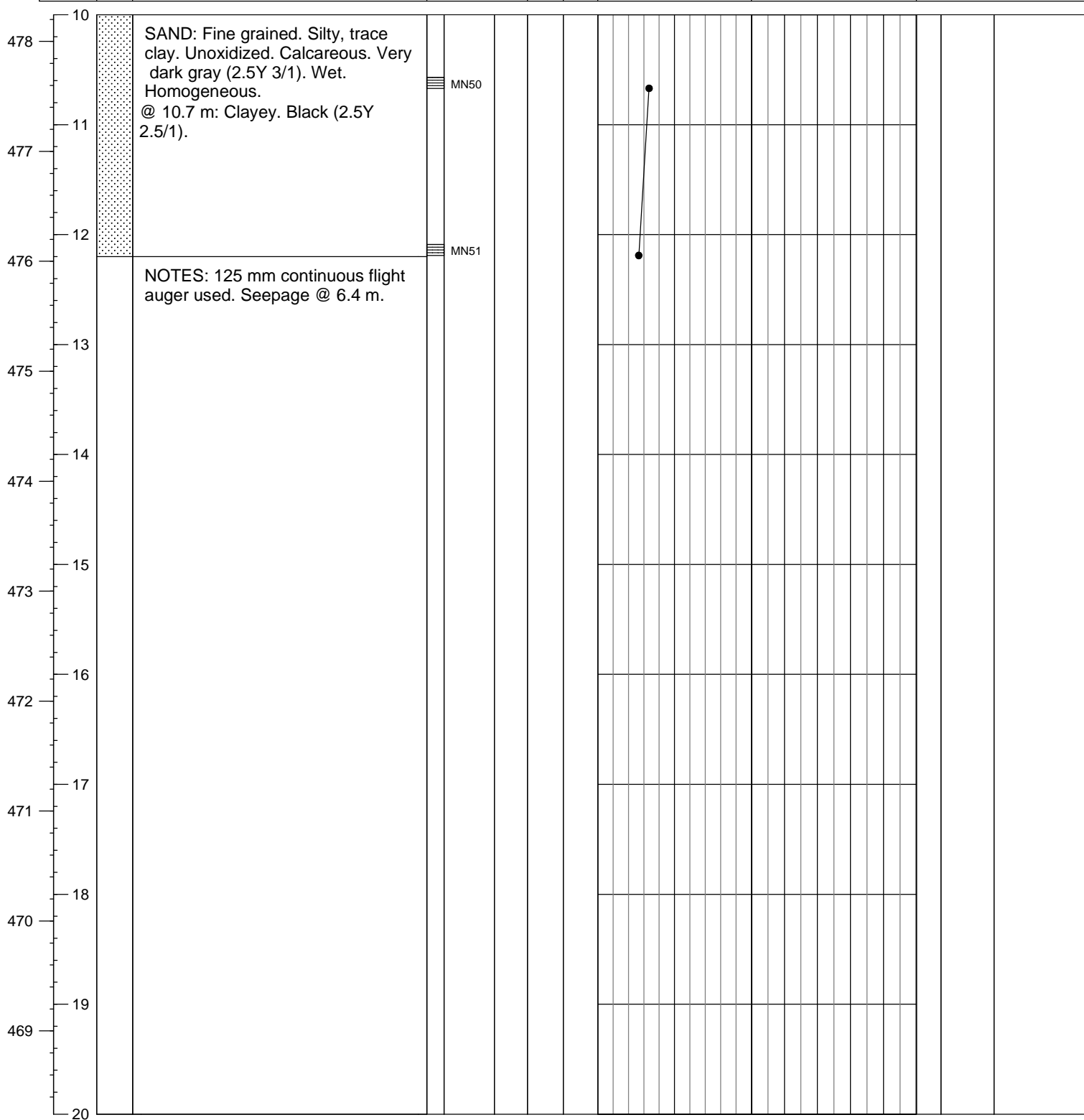
Client: Neil Ketilson Northing: 5,766,577.427 (UTM) Date Drilled: 08 February 2008
Project: Casa Grande Subdivision Easting: 389,262.166 (UTM) Drill: Brat 22
Location: Grasswood, SK Ground Elev.: 488.243 m (Geodetic) Drilling Method: Solid Stem Auger
Project No.: S1607 Top Casing Elev.: Logged by: MN

Elev (m) Depth (m)	Symbol	Soil Description	Sample	USC	% Sulphate	Moisture Content			Shear Strength - kPa Unconf. Pocket Pen. Lab Vane	Piezometer Construction Detail
						Plastic Limit ▲	Natural Moisture ●	Liquid Limit ◆		
			Type No. SPT 'N			0	50	100	1600 2000	





Client:	Neil Ketilson	Northing:	5,766,577.427 (UTM)	Date Drilled:	08 February 2008
Project:	Casa Grande Subdivision	Easting:	389,262.166 (UTM)	Drill:	Brat 22
Location:	Grasswood, SK	Ground Elev.:	488.243 m (Geodetic)	Drilling Method:	Solid Stem Auger
Project No.:	S1607	Top Casing Elev.:		Logged by:	MN

[illegible]

SUMMARY OF SAMPLING AND LABORATORY TEST DATA

SAMPLE				WATER CONTENT	CONSISTENCY				GRADATION				SULPHATE CONTENT	SHEAR STRENGTH			DRY DENSITY
DEPTH	NUMBER	TYPE	RECOVERY		PLASTIC LIMIT	LIQUID LIMIT	PLASTICITY INDEX	USC	GRAVEL	SAND	SILT	CLAY		COMPRESSION TEST	LAB VANE	POCKET PEN	
meters			mm	%	%	%	%		%	%	%	%	%	kPa	kPa	kPa	kg/m ³
0.76	KB1	BAG		4.4													
0.76	KB2	SY	270	5.3			NP	SM	0.0	84.2	16.8				N/A	N/A	
1.52	KB3	SY	300	25.8	23.9	68.2	44.3	CH	0.0	1.4	98.6				175	160	
2.29	KB4	SY	150	9.8			NP	SM	0.0	712.0	28.0				N/A	N/A	
3.05	KB5	SY	200	7.5											N/A	N/A	
4.57	KB6	SY	200	17.8			NP	SM	0.0	85.9	14.1				N/A	N/A	
6.10	KB7	SY	460	25.1													
7.62	KB8	BAG		34.0	23.6	57.9	34.3	CH	0.0	8.3	91.7						
7.92	KB9	BAG		24.4													
9.14	KB10	BAG		38.9	28.7	77.9	49.2	CH	0.0	1.7	98.3						
10.67	KB11	BAG		34.6	24.0	74.6	50.6	CH	0.0	1.8	98.2						
12.19	KB12	BAG		27.0													



Clifton Associates Ltd.
engineering science technology

PROJECT Casa Grande Subdivision
LOCATION Grasswood, Saskatchewan
PROJECT NO. S1607

BORE HOLE NO.

101

SUMMARY OF SAMPLING AND LABORATORY TEST DATA

SAMPLE				WATER CONTENT	CONSISTENCY				GRADATION				SULPHATE CONTENT	SHEAR STRENGTH			DRY DENSITY
DEPTH	NUMBER	TYPE	RECOVERY		PLASTIC LIMIT	LIQUID LIMIT	PLASTICITY INDEX	USC	GRAVEL	SAND	SILT	CLAY		COMPRESSION TEST	LAB VANE	POCKET PEN	
meters			mm	%	%	%	%		%	%	%	%	%	kPa	kPa	kPa	kg/m ³
0.76	KB13	BAG		4.2					0.0	76.7	13.2	10.1					
0.76	KB14	SPT		3.8													
1.52	KB15	BAG		5.1													
2.29	KB16	SPT		4.9													
3.05	KB17	BAG		4.9													
4.57	KB18	SPT		4.7													
6.10	KB19	BAG		4.5													
7.62	KB20	SPT		4.2													
7.92	KB21	BAG		23.3													
9.14	KB22	SPT		26.2													
10.67	KB23	BAG		30.9													
12.19	KB24	SPT		27.5													



Clifton Associates Ltd.
engineering science technology

PROJECT Casa Grande Subdivision
LOCATION Grasswood, Saskatchewan
PROJECT NO. S1607

BORE HOLE NO.

102

SUMMARY OF SAMPLING AND LABORATORY TEST DATA

[illegible]

Clifton Associates Ltd.
engineering science technology

PROJECT Casa Grande Subdivision
LOCATION Grasswood, Saskatchewan
PROJECT NO. S1607

BORE HOLE NO.

103

SUMMARY OF SAMPLING AND LABORATORY TEST DATA

SAMPLE				WATER CONTENT	CONSISTENCY				GRADATION				SULPHATE CONTENT	SHEAR STRENGTH			DRY DENSITY
DEPTH	NUMBER	TYPE	RECOVERY		PLASTIC LIMIT	LIQUID LIMIT	PLASTICITY INDEX	USC	GRAVEL	SAND	SILT	CLAY		COMPRESSION TEST	LAB VANE	POCKET PEN	
meters			mm	%	%	%	%		%	%	%	%	%	kPa	kPa	kPa	kg/m ³
0.76	KB31	BAG		9.2			NP	SM	0.0	55.4	44.6						
1.52	KB32	BAG		11.8	19.5	43.3	23.8	CL	0.0	36.0	64.0						
2.29	KB33	BAG		16.5	18.9	49.1	30.2	CL	0.0	17.7	82.3						
3.05	KB34	BAG		18.6													
3.05	KB35	SY	310	23.4	26.4	63.7	37.3	CH	0.0	1.0	99.0				260+	290+	
4.57	KB36	BAG		13.5													
4.57	KB37	SY	260	9.1			NP	SM	0.0	57.0	43.0				260+	290+	
6.10	KB38	BAG		16.3													
7.62	KB39	BAG		28.7			NP	SM	0.0	77.3	22.7						
9.14	KB40	BAG		25.6													



Clifton Associates Ltd.
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PROJECT Casa Grande Subdivision
LOCATION Grasswood, Saskatchewan
PROJECT NO. S1607

BORE HOLE NO.

104

SUMMARY OF SAMPLING AND LABORATORY TEST DATA

[illegible]

Clifton Associates Ltd.
engineering science technology

PROJECT Casa Grande Subdivision
LOCATION Grasswood, Saskatchewan
PROJECT NO. S1607

BORE HOLE NO.

105

SUMMARY OF SAMPLING AND LABORATORY TEST DATA

[illegible]

PROJECT	Casa Grande Subdivision
LOCATION	Grasswood, Saskatchewan
PROJECT NO.	S1607

BORE HOLE NO.

106

BORE HOLE NO.

106

SUMMARY OF SAMPLING AND LABORATORY TEST DATA

[illegible]

Clifton Associates Ltd.
engineering science technology

PROJECT Casa Grande Subdivision
LOCATION Grasswood, Saskatchewan
PROJECT NO. S1607

BORE HOLE NO.

107

SUMMARY OF SAMPLING AND LABORATORY TEST DATA

SAMPLE				WATER CONTENT	CONSISTENCY				GRADATION				SULPHATE CONTENT	SHEAR STRENGTH			DRY DENSITY
DEPTH	NUMBER	TYPE	RECOVERY		PLASTIC LIMIT	LIQUID LIMIT	PLASTICITY INDEX	USC	GRAVEL	SAND	SILT	CLAY		COMPRESSION TEST	LAB VANE	POCKET PEN	
meters			mm	%	%	%	%		%	%	%	%	%	kPa	kPa	kPa	kg/m ³
0.76	KB60	BAG		26.1													
1.52	KB61	BAG		36.0			NP	SM	0.0	79.2	20.8						
1.52	KB62	SPT		17.9													
2.29	KB63	BAG		6.7			NP	SM	0.0	79.8	20.2						
2.29	KB64	SPT		9.0													
2.29	KB64B	SPT		35.5													
3.05	KB65	BAG		22.4	19.9	33.7	13.8	CL	0.0	20.6	79.4						
3.05	KB66	SPT		16.4													
4.57	KB67	BAG		22.6													
6.10	KB68	BAG		25.4													
6.71	KB69	BAG		35.1	16.4	51.9	37.3	CH	0.0	3.4	96.6						
7.62	KB70	BAG		24.7													
9.14	KB71	BAG		23.0													
10.67	KB72	BAG		25.1													
12.19	KB73	BAG		25.4													



Clifton Associates Ltd.
engineering science technology

PROJECT Casa Grande Subdivision
LOCATION Grasswood, Saskatchewan
PROJECT NO. S1607

BORE HOLE NO.

108

SUMMARY OF SAMPLING AND LABORATORY TEST DATA

SAMPLE				WATER CONTENT	CONSISTENCY				GRADATION				SULPHATE CONTENT	SHEAR STRENGTH			DRY DENSITY
DEPTH	NUMBER	TYPE	RECOVERY		PLASTIC LIMIT	LIQUID LIMIT	PLASTICITY INDEX	USC	GRAVEL	SAND	SILT	CLAY		COMPRESSION TEST	LAB VANE	POCKET PEN	
meters			mm	%	%	%	%		%	%	%	%	%	kPa	kPa	kPa	kg/m ³
0.76	KB74	BAG		6.2													
1.52	KB75	BAG		5.4													
2.29	KB76	BAG		10.4													
3.05	KB77	BAG		30.7													
3.05	KB78	SY	230	23.9											N/A	N/A	
4.57	KB79	BAG		36.1													
4.57	KB80	SY	390	22.5											30	35	
6.10	KB81	BAG		25.8													



Clifton Associates Ltd.
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PROJECT Casa Grande Subdivision
LOCATION Grasswood, Saskatchewan
PROJECT NO. S1607

BORE HOLE NO.

109

SUMMARY OF SAMPLING AND LABORATORY TEST DATA

[illegible]

Clifton Associates Ltd.
engineering science technology

PROJECT Casa Grande Subdivision
LOCATION Grasswood, Saskatchewan
PROJECT NO. S1607

BORE HOLE NO.

110

SUMMARY OF SAMPLING AND LABORATORY TEST DATA

SAMPLE				WATER CONTENT	CONSISTENCY				GRADATION				SULPHATE CONTENT	SHEAR STRENGTH			DRY DENSITY
DEPTH	NUMBER	TYPE	RECOVERY		PLASTIC LIMIT	LIQUID LIMIT	PLASTICITY INDEX	USC	GRAVEL	SAND	SILT	CLAY		COMPRESSION TEST	LAB VANE	POCKET PEN	
meters			mm	%	%	%	%		%	%	%	%	%	kPa	kPa	kPa	kg/m ³
0.76	MN25	BAG		18.0					0.0	74.6	12.6	12.8					
1.52	MN26	BAG		23.7													
2.29	MN27	BAG		24.4													
3.05	MN28	BAG		26.4													
4.57	MN29	BAG		26.2													
6.10	MN30	BAG		36.1													
7.62	MN31	BAG		35.6													
9.14	MN32	BAG		34.8													
10.67	MN33	BAG		34.9													
12.19	MN34	BAG		33.5													
13.72	MN35	BAG		36.0													



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PROJECT Casa Grande Subdivision
LOCATION Grasswood, Saskatchewan
PROJECT NO. S1607

BORE HOLE NO.

111

SUMMARY OF SAMPLING AND LABORATORY TEST DATA

[illegible]

Clifton Associates Ltd.
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PROJECT Casa Grande Subdivision
LOCATION Grasswood, Saskatchewan
PROJECT NO. S1607

BORE HOLE NO.

112

SUMMARY OF SAMPLING AND LABORATORY TEST DATA

[illegible]

Clifton Associates Ltd.
engineering science technology

PROJECT Casa Grande Subdivision
LOCATION Grasswood, Saskatchewan
PROJECT NO. S1607

BORE HOLE NO.

113

SUMMARY OF SAMPLING AND LABORATORY TEST DATA

[illegible]

PROJECT	Casa Grande Subdivision
LOCATION	Grasswood, Saskatchewan
PROJECT NO.	S1607

BORE HOLE NO.

114

BORE HOLE NO.

114

SUMMARY OF SAMPLING AND LABORATORY TEST DATA

SAMPLE				WATER CONTENT	CONSISTENCY				GRADATION				SULPHATE CONTENT	SHEAR STRENGTH			DRY DENSITY
DEPTH	NUMBER	TYPE	RECOVERY		PLASTIC LIMIT	LIQUID LIMIT	PLASTICITY INDEX	USC	GRAVEL	SAND	SILT	CLAY		COMPRESSION TEST	LAB VANE	POCKET PEN	
meters			mm	%	%	%	%		%	%	%	%	%	kPa	kPa	kPa	kg/m ³
0.76	MN58	BAG		6.9									0.01				
1.52	MN59	BAG		22.8									0.14				
2.29	MN60	BAG		33.4													
3.05	MN61	BAG		34.4													
4.57	MN62	BAG		37.7													
6.10	MN63	BAG		32.8													



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PROJECT Casa Grande Subdivision
LOCATION Grasswood, Saskatchewan
PROJECT NO. S1607

BORE HOLE NO.

115

SUMMARY OF SAMPLING AND LABORATORY TEST DATA

SAMPLE				WATER CONTENT	CONSISTENCY				GRADATION				SULPHATE CONTENT	SHEAR STRENGTH			DRY DENSITY
DEPTH	NUMBER	TYPE	RECOVERY		PLASTIC LIMIT	LIQUID LIMIT	PLASTICITY INDEX	USC	GRAVEL	SAND	SILT	CLAY		COMPRESSION TEST	LAB VANE	POCKET PEN	
meters			mm	%	%	%	%		%	%	%	%	%	kPa	kPa	kPa	kg/m ³
0.76	MN1	BAG		19.9									0.74				
1.52	MN2	BAG		23.4									1.11				
2.29	MN3	BAG		29.5													
3.05	MN4	BAG		30.5													
4.57	MN5	BAG		33.4													
6.10	MN6	BAG		30.5													



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PROJECT Casa Grande Subdivision
LOCATION Grasswood, Saskatchewan
PROJECT NO. S1607

BORE HOLE NO.

116

SUMMARY OF SAMPLING AND LABORATORY TEST DATA

[illegible]

Clifton Associates Ltd.
engineering science technology

PROJECT Casa Grande Subdivision
LOCATION Grasswood, Saskatchewan
PROJECT NO. S1607

BORE HOLE NO.

117

SUMMARY OF SAMPLING AND LABORATORY TEST DATA

[illegible]

PROJECT	Casa Grande Subdivision
LOCATION	Grasswood, Saskatchewan
PROJECT NO.	S1607

BORE HOLE NO.

118

BORE HOLE NO.

118

SUMMARY OF SAMPLING AND LABORATORY TEST DATA

SAMPLE				WATER CONTENT	CONSISTENCY				GRADATION				SULPHATE CONTENT	SHEAR STRENGTH			DRY DENSITY
DEPTH	NUMBER	TYPE	RECOVERY		PLASTIC LIMIT	LIQUID LIMIT	PLASTICITY INDEX	USC	GRAVEL	SAND	SILT	CLAY		COMPRESSION TEST	LAB VANE	POCKET PEN	
meters			mm	%	%	%	%		%	%	%	%	%	kPa	kPa	kPa	kg/m ³
0.76	MN42	BAG		10.2													
1.52	MN43	BAG		9.4													
2.29	MN44	BAG		8.6													
2.74	MN44A	SY	220	2.8											N/A	N/A	
3.05	MN45	BAG		3.7													
4.57	MN46	BAG		23.9													
6.10	MN47	BAG		30.1													
7.62	MN48	BAG		28.0													
9.14	MN49	BAG		26.8													
10.67	MN50	BAG		33.4													
12.19	MN51	BAG		26.7													



Clifton Associates Ltd.
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PROJECT Casa Grande Subdivision
LOCATION Grasswood, Saskatchewan
PROJECT NO. S1607

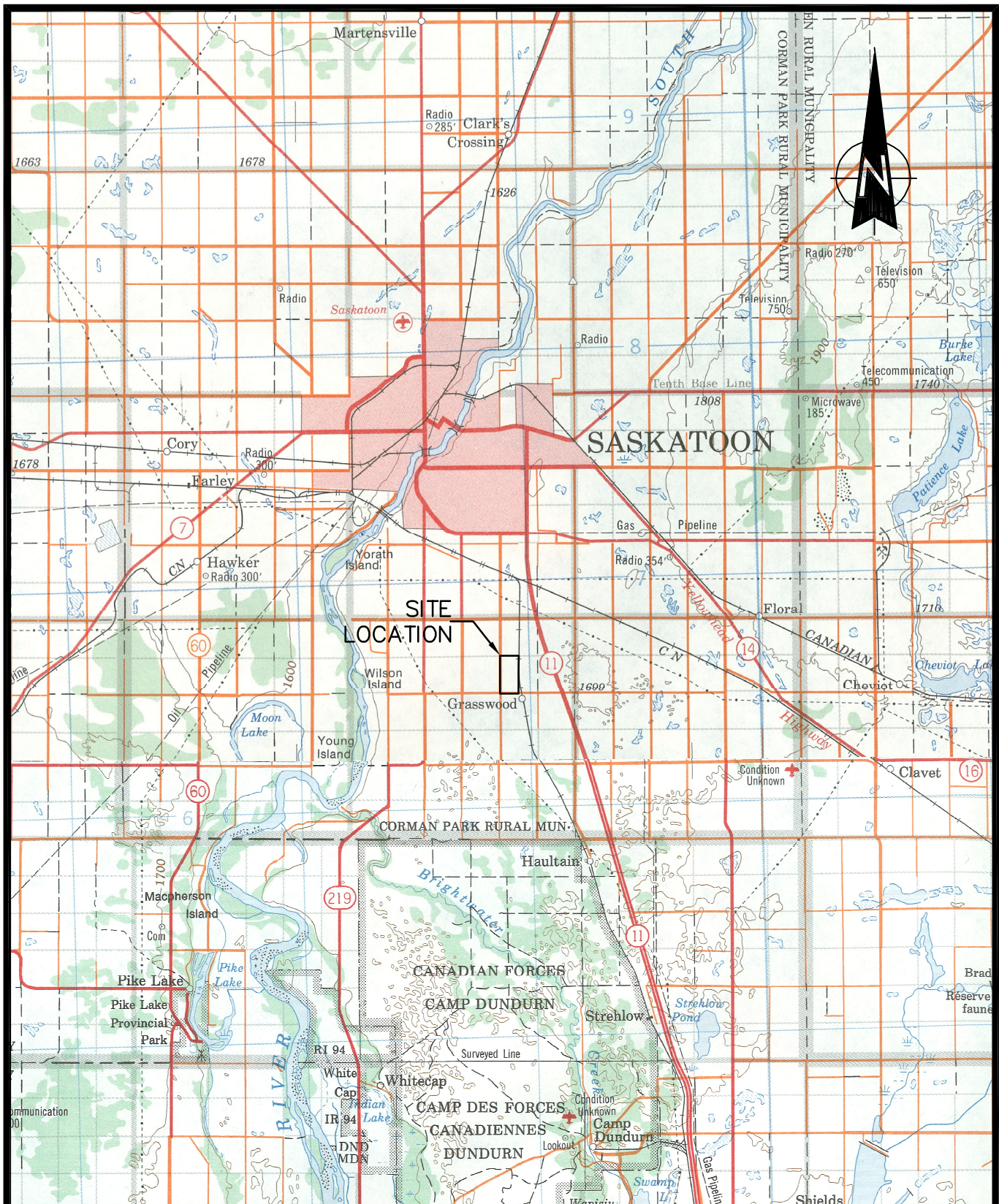
BORE HOLE NO.

119



Clifton Associates Ltd.
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Drawings



DATE 14/03/08	DRAWN BY LLT	APPROVED BY	SCALE NTS	FILE NO. S1607001	DRAWING NO. S1607-01
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engineering science technology

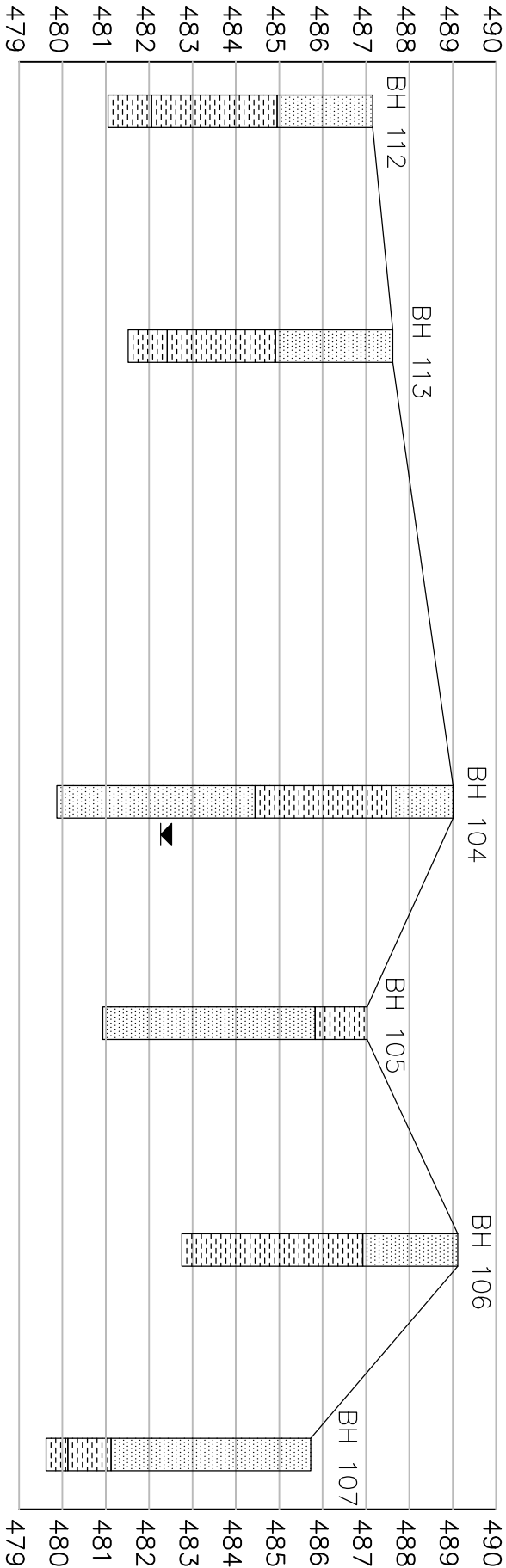
CLIENT	NEIL KETILSON
PROJECT	CASA GRANDE SUBDIVISION
TITLE	SITE LOCATION PLAN

LEGEND:

CLAY


SILT

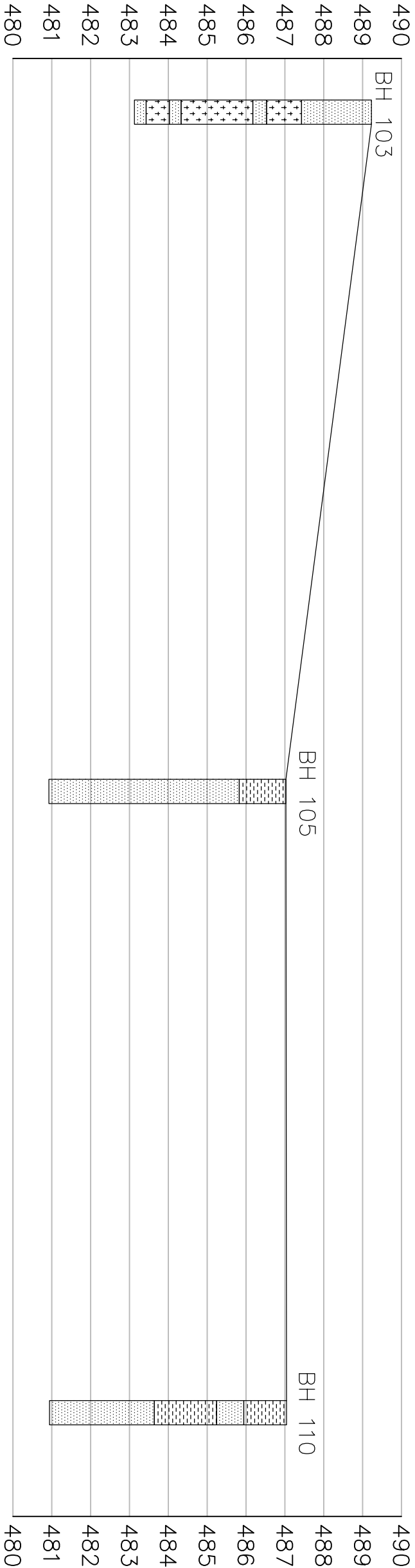
SAND



SCALE
VERTICAL: 1:375,000
HORIZONTAL: 1:7,500

DRAWING REVISIONS			
6			
5			
4			
3			
2			
1			
NO.	DD/MM/YY	DESCRIPTION	BY

 <div>Clifton Associates Ltd. engineering science technology</div>		CLIENT	
		NEIL KETILSON	
		PROJECT TITLE	
		CASA GRANDE SUBDIVISION	
		DRAWING TITLE	
		CROSS SECTION A-A' (NORTH-SOUTH)	
PROJECT NO.	S1607	FILE NO.	S1607/001
DATE	20/03/08	SCALE	AS SHOWN
DRAWN	LLT	CHECKED	
		DWG. NO. S1607-03	



SCALE
VERTICAL: 1:50,000
HORIZONTAL: 1:2,500


LEGEND:

CLAY

SILT

SAND

DRAWING REVISIONS			
6			
5			
4			
3			
2			
1			
NO.	DD/MM/YY	DESCRIPTION	BY

		Clifton Associates Ltd. engineering science technology	
CLIENT			
NEIL KETILSON			
PROJECT TITLE			
CASA GRANDE SUBDIVISION			
DRAWING TITLE			
CROSS SECTION B-B' (WEST-EAST)			
PROJECT NO.	FILE NO.		
S1607	S1607/001		
DATE	SCALE	DWG. NO.	
20/03/08	AS SHOWN		
DRAWN	CHECKED	S1607-04	
LT			



Clifton Associates Ltd.
engineering science technology

Appendix A



Environmental Division

ANALYTICAL REPORT

CLIFTON ASSOCIATES LTD

ATTN: KIM BONNEAU

2120 AIRPORT DR.

SASKATOON SK S7L 6M6

Reported On: 27-FEB-08 03:23 PM

Lab Work Order #: **L605119**

Date Received: **26-FEB-08**

Project P.O. #:

Job Reference: S1607

Legal Site Desc:

CofC Numbers: C061070

Other Information:

Comments:

NICK PIDSKALNY
General Manager, Saskatoon

For any questions about this report please contact your Account Manager:

RAECHELLE KREESE

THIS REPORT SHALL NOT BE REPRODUCED EXCEPT IN FULL WITHOUT THE WRITTEN AUTHORITY OF THE LABORATORY.
ALL SAMPLES WILL BE DISPOSED OF AFTER 30 DAYS FOLLOWING ANALYSIS. PLEASE CONTACT THE LAB IF YOU
REQUIRE ADDITIONAL SAMPLE STORAGE TIME.

ALS Canada Ltd. (formerly ETL Chemspec Analytical Ltd.)
Part of the **ALS Laboratory Group**

#819-58th St E., Saskatoon, SK S7K 6X5

Phone: +1 306 668 8370 Fax: +1 306 668 8383 www.alsglobal.com

A Campbell Brothers Limited Company

ALS LABORATORY GROUP ANALYTICAL REPORT

Sample Details/Parameters		Result	Qualifier*	D.L.	Units	Extracted	Analyzed	By	Batch
L605119-1 BH 101 Sampled By: NOT PROVIDED on 25-FEB-08 @ 15:00 Matrix: WATER									
Routine Water Analysis									
Alkalinity, Total									
Alkalinity, Total (as CaCO3)		287		5	mg/L	26-FEB-08	26-FEB-08	ANT	R635110
Bicarbonate (HCO3)		350		5	mg/L	26-FEB-08	26-FEB-08	ANT	R635110
Hydroxide (OH)		<5		5	mg/L	26-FEB-08	26-FEB-08	ANT	R635110
Carbonate (CO3)		<5		5	mg/L	26-FEB-08	26-FEB-08	ANT	R635110
Chloride (Cl)		90		1	mg/L	26-FEB-08	26-FEB-08	BFE	R634984
ICP Cations									
Calcium (Ca)		148		1	mg/L	27-FEB-08	27-FEB-08	DAD	R635292
Potassium (K)		4.2		0.1	mg/L	27-FEB-08	27-FEB-08	DAD	R635292
Magnesium (Mg)		73.4		0.1	mg/L	27-FEB-08	27-FEB-08	DAD	R635292
Sodium (Na)		21		1	mg/L	27-FEB-08	27-FEB-08	DAD	R635292
Sulfate (SO4)		263		4	mg/L	27-FEB-08	27-FEB-08	DAD	R635292
Ion Balance Calculation									
Ion Balance		104			%		27-FEB-08		
TDS (Calculated)		783			mg/L		27-FEB-08		
Hardness (as CaCO3)		672			mg/L		27-FEB-08		
Nitrate, Nitrite and Nitrate+Nitrite-N									
Nitrate-N		2.5		0.1	mg/L	26-FEB-08	26-FEB-08	BFE	R635054
Nitrite-N		0.08		0.05	mg/L	26-FEB-08	26-FEB-08	BFE	R635054
Nitrate+Nitrite-N		2.6		0.1	mg/L	26-FEB-08	26-FEB-08	BFE	R635054
pH and Conductivity									
pH		7.5		0.1	pH	26-FEB-08	26-FEB-08	CMF	R635197
Conductivity (EC)		1220		10	uS/cm	26-FEB-08	26-FEB-08	CMF	R635197
L605119-2 BH 108 Sampled By: NOT PROVIDED on 25-FEB-08 @ 15:30 Matrix: WATER									
Routine Water Analysis									
Alkalinity, Total									
Alkalinity, Total (as CaCO3)		308		5	mg/L	26-FEB-08	26-FEB-08	ANT	R635110
Bicarbonate (HCO3)		376		5	mg/L	26-FEB-08	26-FEB-08	ANT	R635110
Hydroxide (OH)		<5		5	mg/L	26-FEB-08	26-FEB-08	ANT	R635110
Carbonate (CO3)		<5		5	mg/L	26-FEB-08	26-FEB-08	ANT	R635110
Chloride (Cl)		6		1	mg/L	26-FEB-08	26-FEB-08	BFE	R634984
ICP Cations									
Calcium (Ca)		91		1	mg/L	27-FEB-08	27-FEB-08	DAD	R635292
Potassium (K)		3.2		0.1	mg/L	27-FEB-08	27-FEB-08	DAD	R635292
Magnesium (Mg)		24.3		0.1	mg/L	27-FEB-08	27-FEB-08	DAD	R635292
Sodium (Na)		15		1	mg/L	27-FEB-08	27-FEB-08	DAD	R635292
Sulfate (SO4)		42		4	mg/L	27-FEB-08	27-FEB-08	DAD	R635292
Ion Balance Calculation									
Ion Balance		101			%		27-FEB-08		
TDS (Calculated)		367			mg/L		27-FEB-08		
Hardness (as CaCO3)		327			mg/L		27-FEB-08		
Nitrate, Nitrite and Nitrate+Nitrite-N									
Nitrate-N		<0.1		0.1	mg/L	26-FEB-08	26-FEB-08	BFE	R635054
Nitrite-N		<0.05		0.05	mg/L	26-FEB-08	26-FEB-08	BFE	R635054
Nitrate+Nitrite-N		0.1		0.1	mg/L	26-FEB-08	26-FEB-08	BFE	R635054
pH and Conductivity									
pH		7.3		0.1	pH	26-FEB-08	26-FEB-08	CMF	R635197
Conductivity (EC)		640		10	uS/cm	26-FEB-08	26-FEB-08	CMF	R635197

ALS LABORATORY GROUP ANALYTICAL REPORT

Sample Details/Parameters	Result	Qualifier*	D.L.	Units	Extracted	Analyzed	By	Batch
L605119-2 BH 108 Sampled By: NOT PROVIDED on 25-FEB-08 @ 15:30 Matrix: WATER Routine Water Analysis								
* Refer to Referenced Information for Qualifiers (if any) and Methodology.								

Reference Information

Methods Listed (if applicable):

ALS Test Code	Matrix	Test Description	Preparation Method Reference(Based On)	Analytical Method Reference(Based On)
ALK-TOT-SK	Water	Alkalinity, Total		APHA 2320 B-Auto-Pot. Titration
Alkalinity is determined by a titration of an aliquot with standardized acid solution to a pH of 4.5. Total alkalinity, bicarbonate, carbonate(if present) and hydroxide(if present) also reported.				
Reference Greenberg, Arnold E., Cleseri, Lenore S., Eaton, Andrew D., Standard Methods For The Examination of Water and Wastewater, 18th Edition, 1992, Method 2320B.				
CL-SK	Water	Chloride (Cl)		APHA 4500 CL-E
Chloride in the extract is determined colorimetrically at 660 nm by complexation with mercury (II) thiocyanate. In the colorimetric method, chloride (Cl-) displaces thiocyanate which, in the presence of ferric iron, forms a highly colored ferric thiocyanate complex.				
Reference Greenberg, Arnold E., Cleseri, Lenore S., Eaton, Andrew D., Standard Methods For The Examination of Water and Wastewater, 18th Edition, 1992, Method 4500Cl-E.				
ETL-ROUTINE-ICP-SK	Water	ICP Cations		APHA 3120 B-ICP-OES
These ions are determined directly y ICP-OES.				
Reference Greenberg, Arnold E., Cleseri, Lenore S., Eaton, Andrew D., Standard Methods For The Examination of Water and Wastewater, 18th Edition, 1992, Method 3120B.				
IONBALANCE-SK	Water	Ion Balance Calculation		APHA 1030E
N2/N3-SK	Water	Nitrate, Nitrite and Nitrate+Nitrite-N		APHA 4500 NO3F
Nitrate is quantitatively reduced to nitrite by passage of the sample through a copperized cadmium column. The nitrite (reduced nitrate plus original nitrite) is then determined by diazotizing with sulfanilamide followed by coupling with N-(1-naphthyl)ethylenediamine dihydrochloride. The resulting water-soluble dye has a magenta color, which is measured at 520nm. Original nitrite can also be determined by removing the cadmium column and following the same procedure. Nitrate-N, Nitrite-N and NO3+NO2-N are reported.				
Reference Greenberg, Arnold E., Cleseri, Lenore S., Eaton, Andrew D., Standard Methods For The Examination of Water and Wastewater, 18th Edition, 1992, Method 4500NO3-F.				
PH/EC-SK	Water	pH and Conductivity		APHA 4500-H, 2510

** Laboratory Methods employed follow in-house procedures, which are generally based on nationally or internationally accepted methodologies.

Chain of Custody numbers:

C061070

The last two letters of the above test code(s) indicate the laboratory that performed analytical analysis for that test. Refer to the list below:

Laboratory Definition Code	Laboratory Location	Laboratory Definition Code	Laboratory Location
SK	ALS LABORATORY GROUP - SASKATOON, SASKATCHEWAN, CANADA		

Reference Information

GLOSSARY OF REPORT TERMS

Surr - A surrogate is an organic compound that is similar to the target analyte(s) in chemical composition and behavior but not normally detected in environmental samples. Prior to sample processing, samples are fortified with one or more surrogate compounds.

The reported surrogate recovery value provides a measure of method efficiency. The Laboratory control limits are determined under column heading *D.L.*

mg/kg (units) - unit of concentration based on mass, parts per million.

mg/L (units) - unit of concentration based on volume, parts per million.

< - Less than.

D.L. - The reporting limit.

N/A - Result not available. Refer to qualifier code and definition for explanation.

Test results reported relate only to the samples as received by the laboratory.

UNLESS OTHERWISE STATED, ALL SAMPLES WERE RECEIVED IN ACCEPTABLE CONDITION.

UNLESS OTHERWISE STATED, SAMPLES ARE NOT CORRECTED FOR CLIENT FIELD BLANKS.

Although test results are generated under strict QA/QC protocols, any unsigned test reports, faxes, or emails are considered preliminary.

ALS Laboratory Group has an extensive QA/QC program where all analytical data reported is analyzed using approved referenced procedures followed by checks and reviews by senior managers and quality assurance personnel. However, since the results are obtained from chemical measurements and thus cannot be guaranteed, ALS Laboratory Group assumes no liability for the use or interpretation of the results.

[illegible]

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